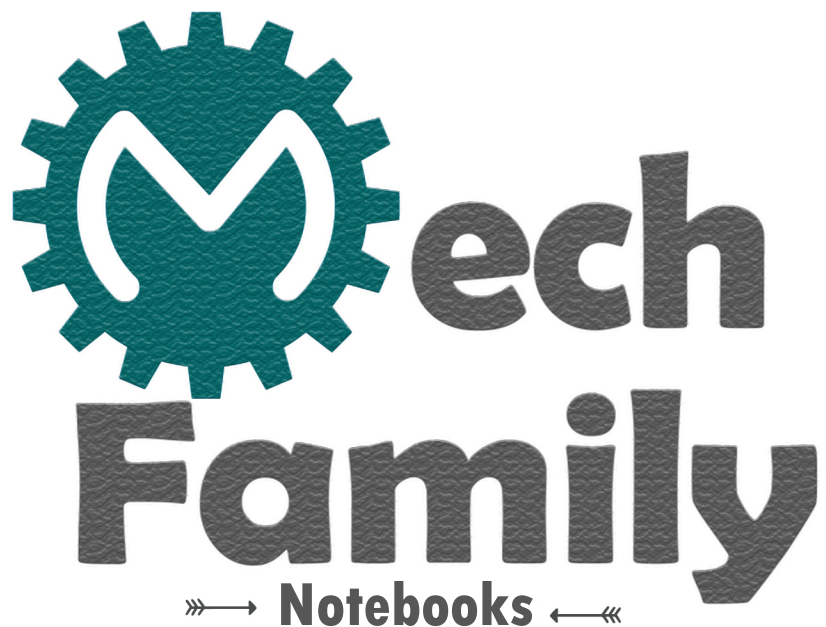


# **Vibrations**

**Dr. Basem Bdoor**

**1st Semester 2017**



# Vibrations

Dr. Basem Al. bdour

1<sup>st</sup> Semester (2017-2018)



25/9/2017

## No. Introduction

Physics Fundamentals and science

+ Mathematics (tools)

Application

→ Civil

→ Mechanical

→ Architecture

\*\*

هاي الفيزياء

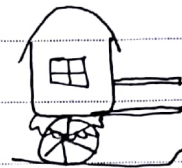
عبارة عن

وينا ما االف

وحيك بتلا في

Vibrations

→ first Application was at horse Cart  
as the cart was held by leaf Spring  
as it became flexible element and  
absorb the effect of road irregularities



\* Tire (Rubber with Air inside) → flexible elements

→ Thermal / Fluids

\* Mechanical

→ Applied Mechanics

→ strength

→ Statics

→ Dynamics

→ Design

→ Machines

\* FYI: Electric Motor efficiency =  $\frac{\text{Output (Mech)}}{\text{Input (elect)}}$   

$$= \frac{T * \omega}{I * V}$$

→ since efficiency is not 100%

that means there is energy losses  
like the Vibration energy

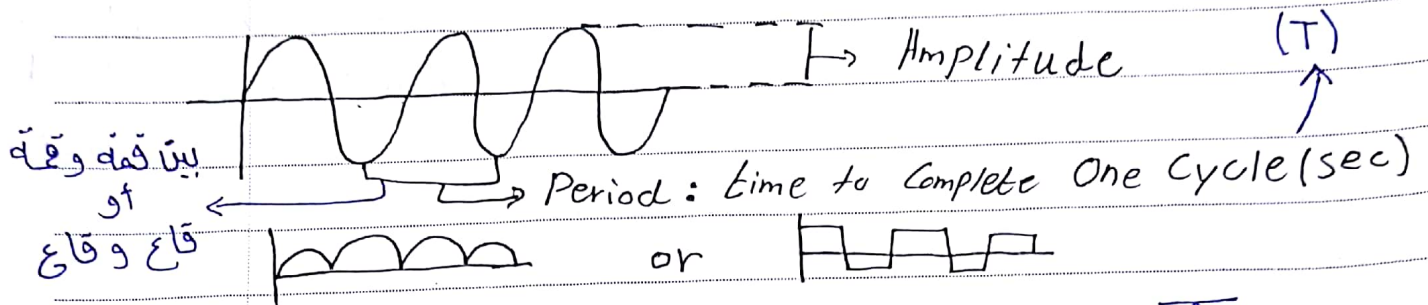
all of them  
are related  
to

→ Tacoma bridge Phenomena! Mech. Vibrations!  
↳ big failure due to vibration  
created by wind at certain speed.

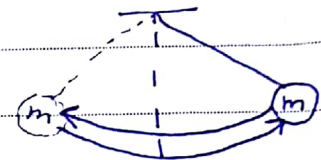
\* Vibration needs to be considered else ⇒ no comfort  
⇒ Health Problem  
Hearing is Vibration, microphone is Vibration!, Sound isolation!,

Vibration: repeated motion (positional) at period of time!

⇒ it can be represented by sinusoidal wave!



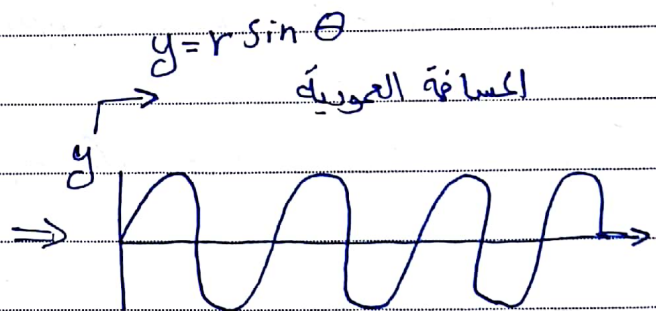
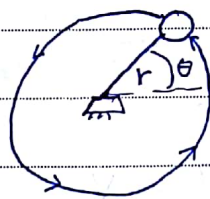
example: The Pendulum Motion



in electric field → Period is  $\frac{1}{50000}$  Sec  
(hertz)

\* Frequency (Hz) =  $\frac{1}{T}$   
or  
cycle per sec (cps)

\* example:



$$\phi = \omega t = 2\pi f t$$

$$\omega = 2\pi f$$



27/9/2017

\* The material is available on website (Slide share.net)

↳ Search in mechanical vibration Slide Share

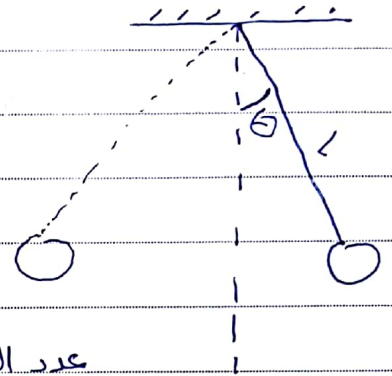
\* Storing Potential energy Types : ① gravity (height)  
② elasticity (Springs)

elasticity ↔ Stiffness

\* Simplest Type of Free vibration → The pendulum.

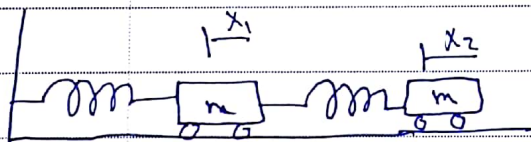
KE is Zero at Maximum height

$mgl ( )$



\* The 3 elements of vib system: ① mass  
② Spring  
③ damper

\* degree of freedom → عدد ال Parameters التي يحتاجها  
ما في النظام من الجسيمات



\* 2 degree of freedom

( 2 Single and independent  
degree of freedom  
for each part )

## Vibration Classification

\* Free or Forced

Damped & Undamped

Linear and Non-Linear

Deterministic or Random

↓  
Differential Equation!

(Predictable / not Predictable)

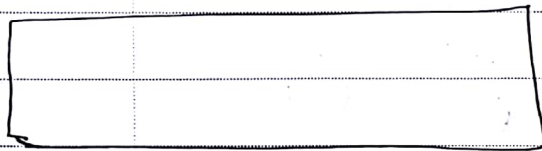
والتحليل يكون

\* There is Assumption must be taken, then Formulate and Analysing and Differentiation in order to find the Vibration Results (system Characteristic)

\* Spring Element:

\* For a Cantilever beam:

the deflection at Free end is



→ take the Force and treat like a spring so you can find the Spring Constant  $K$

end

Springs in series  $\rightarrow \frac{1}{K_{eq}} = \frac{1}{K_1} + \frac{1}{K_2} + \frac{1}{K_3} \dots$

Springs in parallel  $\rightarrow K_{eq} = K_1 + K_2 + K_3$

$$F = K \times \delta$$

من ملاحظة الـ

يمكن فهم لييش

(Check the Slides)



2/10/2017

No.

- Vibration: Motion that repeats it self at certain pattern

- Free vibration: Initial disturbance (excitation)  
Vibrates freely, Continuous exchange between (KE & PE)

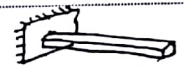
\* For the same material, Area, Cross-section  
the speed of Vibration doesn't Change.

\* Change in Length  $\rightsquigarrow$  Change in Stiffness (K)

- Forced vibration: under Continuous input from the outside of the system

- Degree of Freedom (DoF):

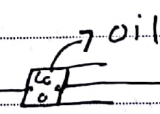
- Continuous system ( $\infty$  DoFs) ex: Beam



DoF يعني عدد الـ Coordinates اللي يحتاجها عشان

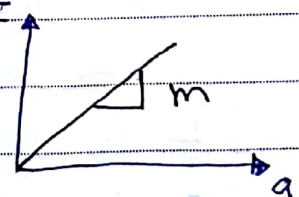
أقدر أوصف حركة جسم (X, Y, Z, ...)

- Discrete system (Finite DoF's) ex: Single DoF

- Damping: - Damped (oil or Friction)   
- Undamped  $\rightsquigarrow$  No Energy dissipation

\* The system that we deal with is:  $F$

□ Mass element  $\rightsquigarrow$  Store KE



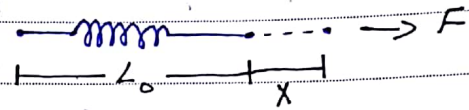
\* Inertia is Body Resistance to accelerate.  $F = ma$  Linear acceleration

- For Angular acceleration  $\Rightarrow M = J \ddot{\theta} = J \alpha \text{ (rad/s}^2\text{)}$

\* Ideal point mass, no dimensions, }  $m = m_1 + m_2 + \dots + m_n$   
 No spring effect, No damping

## 2 Spring element : (Store PE)

$\rightarrow$  transmits all forces

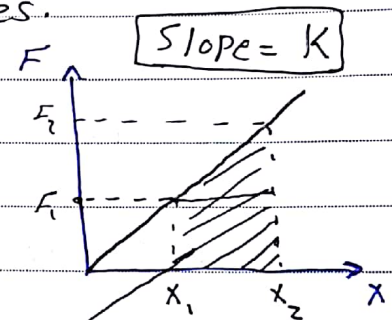


- gives flexibility for suspension system and resist the system inputs like earth quakes.

- Slope =  $K$  = Stiffness

- massless
- linear
- No damping

\* Linear و Non-linear Curve  
 Linear و Non-linear hardening (deflection) (dx) (dx)



- So in this course we will deal with the linear region only for development and predict system behavior.

- The area is Energy Stored in Spring

$$E = \frac{1}{2} K x^2$$

$$F = K x$$

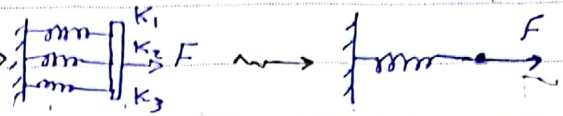
integrate  
 from Force to energy

"hardening"



## \* Spring Connections :

- Spring in Parallel



$$F = F_1 + F_2 + \dots + F_n \leadsto K_{eq} * x = k_1 x_1 + k_2 x_2 + \dots + k_n x_n \quad \left\{ \begin{array}{l} x = x_1 = x_2 \\ \dots \end{array} \right.$$

$$K_{eq} = \sum k_n$$

- Spring in Series :



$$x_{tot} = x_1 + x_2 + x_3 + \dots + x_n \leadsto \frac{F}{K_{eq}} = \frac{F}{k_1} + \frac{F}{k_2} + \dots + \frac{F}{k_n}$$

\* يعني عكس القابضات الي  
بالكهرباء

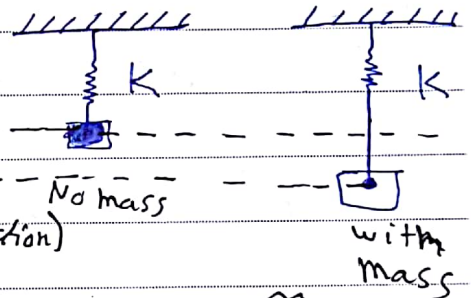
$$K_{eq} = \sum \frac{1}{k_n}$$

\* الحالة اسهل

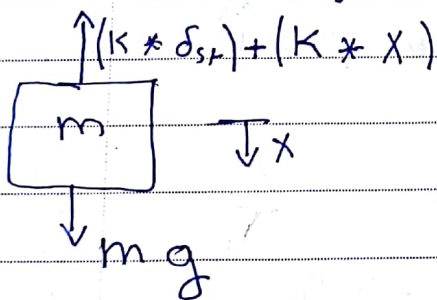
"Static equilibrium position"

$$mg = K \delta_{st}$$

Static deflection



\* F.B.D & apply Newton's 2<sup>nd</sup> Law:

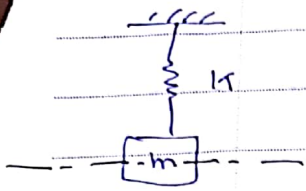


$$\sum F_x = m \ddot{x} = m g - K \delta_{st} - K x$$

$$m \ddot{x} + K x = 0$$

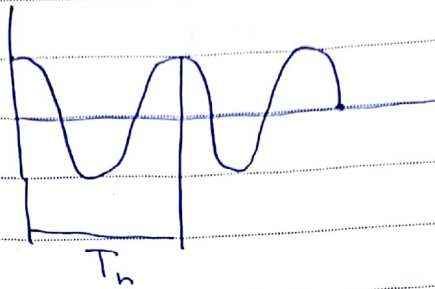
(2<sup>nd</sup> ODE, homogeneous, Constant Coefficient)

Initial Condition  
are needed  $(x_0, \dot{x}_0)$



"Static equilibrium Position"

$$K \delta_{st} = mg$$



$$m \ddot{x} + k x = 0 \quad \rightarrow \quad x(0) = x_0, \dot{x}(0) = v_0$$

→ Assume  $x(t) = A e^{\lambda t}$  } From the solution of 2<sup>nd</sup> ODE, homogeneous, Linear

↓

$\dot{x}(t) = A \lambda e^{\lambda t}$

$\ddot{x}(t) = A \lambda^2 e^{\lambda t}$

Substitute in equation

$$m \lambda^2 A e^{\lambda t} + k A e^{\lambda t} = 0$$

$$\downarrow$$

$$(m \lambda^2 + k) A e^{\lambda t} = 0 \rightarrow m \lambda^2 + k = 0$$

$$\lambda = \pm \sqrt{\frac{-k}{m}} = \pm i \sqrt{\frac{k}{m}}$$

$$\Rightarrow x(t) = B_1 \cos\left(\sqrt{\frac{k}{m}} t\right) + B_2 \sin\left(\sqrt{\frac{k}{m}} t\right)$$

The Natural Frequency ( $\omega_n$ ) =  $\frac{2\pi}{T_n}$

$$= 2\pi f$$

$$f = \frac{1}{T_n}$$

natural period

(Hz)

cos & sin بـ  $\omega_n$  و  $t$  بالترتيب

$$e^{i\alpha} = \cos \alpha + i \sin \alpha$$

← Euler's Identity



No.

$$X_s(t) = B_1 \cos(\omega_n t) + B_2 \sin(\omega_n t)$$

$$= B_1 \cos\left(\frac{2\pi}{T} t\right) + B_2 \sin\left(\frac{2\pi}{T} t\right)$$

$$= B_1 \cos 2\pi f t + B_2 \sin(2\pi f t)$$

\*  $B_1$  and  $B_2$  are found by The initial Condition  ~~$X(0) = X_0$~~

$$\rightarrow B_1 X(0) = B_1 + 0 = X_0$$

$$\begin{aligned} \dot{X}(0) &= V_0 \\ X(0) &= X_0 \end{aligned}$$

$$\rightarrow \boxed{B_1 = X_0}$$

$$\rightarrow \dot{X}(0) = 0 + B_2 \omega_n = V_0$$

$$\boxed{B_2 = \frac{V_0}{\omega_n}}$$

كل مدار الكلي

Complementary Solution

$\rightarrow$  so The general Solution is

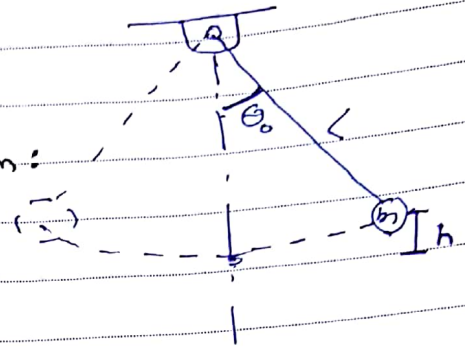
$$X_{gs} = X_{cs} + \overset{\substack{\text{Particular} \\ \text{Integral}}}{\underset{PI}{X}}$$

$$\boxed{X_{gs} = X_{cs} = X_0 \cos \omega_n t + \frac{V_0}{\omega_n} \sin \omega_n t}$$

مدار بطول  $\omega_n$  يكون  
الطرف الثاني (البينا)  
ما يساوي صفر

## Simple Pendulum Example:

let's take a Free body diagram:

\* we need to use Equation with  
The Variable  $\theta$ 

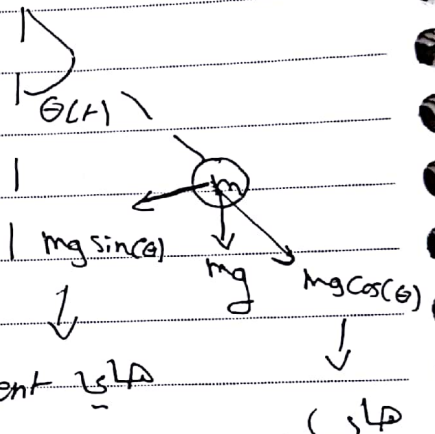
\* by Parallel axis Theorem

 $J_0$  (mass moment of inertia)

F.B.D

here

$$J_0 = mL^2$$

(For Simple  
Pendulum) $\Rightarrow$  The Equation of Motion:

$$mL^2 \ddot{\theta} + mgL \sin(\theta) = 0$$

2<sup>nd</sup> ODE, Non-linear (sin), homogeneous

Normal

$\leadsto$  By Taylor Expansion and Assuming  $\sin \theta \approx \theta$   
(Small Angle)

$$\rightarrow mL^2 \ddot{\theta} + mgL \theta = 0$$

$$\downarrow$$

$$L \ddot{\theta} + g \theta = 0 \quad ; \quad g = 9.81 \text{ m/s}^2$$



Recall the Equation of the Previous Example:

$$m \ddot{x} + kx = 0 \rightsquigarrow \ddot{x} + \frac{k}{m} x = 0 \rightsquigarrow \ddot{x} + \omega_n^2 x = 0$$

بدنا نحل نفس الإشياء بحالة الـ Pendulum

$$\ddot{\theta} + \frac{g}{L} \theta = 0$$

$$\omega_n^2 \rightsquigarrow \omega_n = \sqrt{\frac{g}{L}}$$

\* إذا الإشي الوحيد اللي يتحكم بالمعادلة هو الطول (L)  
 \* بما اننا نفس المعادلة فنحل بنفس طريقة سؤال الـ Spring

$$\Rightarrow \theta(t) = \theta_0 \cos(\omega_n t) + \frac{\dot{\theta}_0}{\omega_n} \sin(\omega_n t) \quad \omega_n = \sqrt{\frac{g}{L}}$$

\* Energy Conservation of Energy (طريقة ثانية للحل)

$$P.E = \frac{1}{2} k x^2, K.E = \frac{1}{2} m \dot{x}^2$$

$$\text{So} \rightarrow \frac{1}{2} m \dot{x}^2 + \frac{1}{2} k x^2 = \text{Constant!}$$

↓ taking The derivatives

$$m \cancel{\dot{x}} \ddot{x} + k x \cancel{\dot{x}} = 0 \rightsquigarrow \boxed{m \ddot{x} + k x = 0}$$

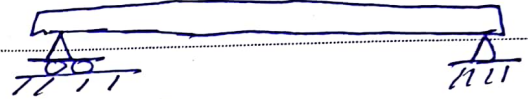
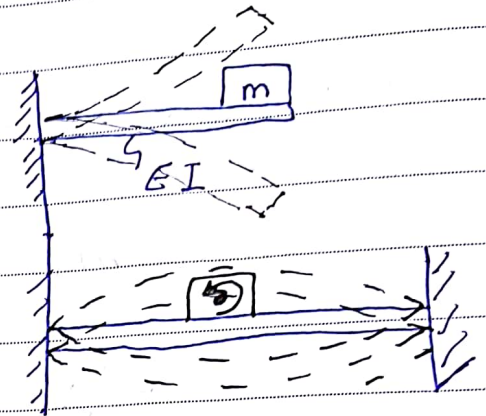
Some Procedure for simple pendulum:

$$K.E = \frac{1}{2} I \dot{\theta}^2, P.E = mg(\text{height}) = mg(L - L \cos \theta)$$

$$\Rightarrow \frac{d}{dt} \left( \frac{1}{2} I \dot{\theta}^2 + mg(L - L \cos \theta) \right) = \text{Constant}$$

No.

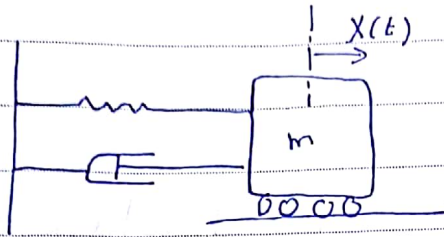
Example  $\rightarrow$  Cantilever Beam



\* Quiz  $\rightarrow$  next week, read the first Chapter (1)

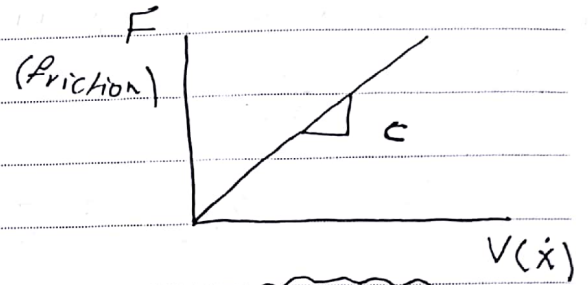
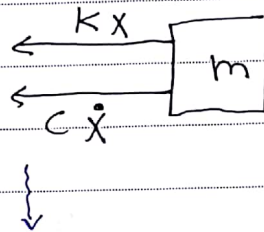


## Free Vibration of SDOF "Viscous Damping"



Damper يكون في عند  
بتأثر بقوة احتكاك بغير  
فقط على السرعة  $\dot{x}$

drawing The  
F. B. D



$$c = \frac{F_d}{\dot{x}}$$

$$\sum F_x = m \ddot{x}$$

$$m \ddot{x} = -c \dot{x} - kx$$

$$x(0) = x_0, \quad \dot{x}(0) = \dot{x}_0 \rightarrow \text{I.C.'s}$$

$\rightarrow$  2<sup>nd</sup> O.D.E with Constant Coefficients, homogeneous

assume 
$$\begin{aligned} x(t) &= A e^{\lambda t} \\ \dot{x}(t) &= A \lambda e^{\lambda t} \\ \ddot{x}(t) &= A \lambda^2 e^{\lambda t} \end{aligned} \quad \rightarrow \text{Substitute in the previous Equation}$$

$\rightarrow$  The Characteristic equation  $\rightarrow m \lambda^2 + c \lambda + k = 0$

The roots are

$$\lambda_{1,2} = \frac{-c \pm \sqrt{c^2 - 4mk}}{2m}$$

$$\frac{-c}{2m} \pm \sqrt{\frac{c^2 - 4mk}{4m^2}}$$

So the roots are:

$$\frac{-c}{2m} \pm \sqrt{\frac{c^2}{4m} - \frac{km}{m}}$$

\* حسب ما في القابلة في عند 3 حالات (Real, Zero, Imaginary) ما راجع البذور ( $>0$ ,  $=0$ ,  $<0$ )

□  $\frac{c^2}{4m} - \frac{k}{m} = 0$ , (Critically Damped)

So  $\lambda_{1,2} = \frac{-c}{2m}$

$$\frac{c^2}{4m} - k = 0 \rightarrow c_{cr}^2 = 4mk$$

Critical Damping ( $c_{cr}$ )  $\leftarrow c_{cr} = 2\sqrt{mk}$

\* Now we will define a new parameter  $\zeta$   
Damping Ratio  $\leftarrow$

$$\zeta = \frac{c}{c_{cr}} \rightarrow \text{dimensionless!}$$

Based on  $\zeta$  we have 4 cases

- if  $c > c_{cr} \rightarrow \zeta > 1 \rightarrow$  Over damped  
 $c = c_{cr} \rightarrow \zeta = 1 \rightarrow$  critically damped  
 $c < c_{cr} \rightarrow \zeta < 1 \rightarrow$  Under damped  
 $c = 0 \rightarrow \zeta = 0 \rightarrow$  Undamped



$$\rightarrow C = \zeta C_{cr} = \zeta * 2 * \sqrt{mk}$$

$$\lambda_{1,2} = -\zeta \sqrt{\frac{mk}{m}} = -\zeta \sqrt{\frac{mk}{m}}$$

پایه ای اگر 0  
 $\sqrt{\frac{c^2}{4m} - 4mk} = 0$

$$\text{So } \lambda_{1,2} = -\zeta \sqrt{\frac{k}{m}} \pm \sqrt{\frac{\zeta^2 k}{m} - \frac{k}{m}}$$

$$= -\zeta \omega_n \pm \omega_n \sqrt{\zeta^2 - 1}$$

و پای 0  
 بکون ما را قبل  
 الجز را مساوی می

So Based on this equation and the 4 cases of  $\zeta$ :

① For undamped case:  $\zeta = 0$ ,  $\lambda_{1,2} = \pm i\omega_n$

$$X(t) = A \cos(\omega_n t) + B \sin(\omega_n t)$$

② For under damped case:  $0 < \zeta < 1$ ,  $\lambda_{1,2} = -\zeta \omega_n \pm i \omega_n \sqrt{1 - \zeta^2}$

$\rightarrow$  here we define new parameter:

$$\text{Damping frequency } (\omega_d) = \omega_n \sqrt{1 - \zeta^2}$$

(damped natural frequency)

$$X(t) = A_1 e^{(-\omega_n + i\omega_d)t} + A_2 e^{(-\omega_n - i\omega_d)t} \quad \leftarrow \lambda_{1,2} = -\zeta \omega_n \pm i \omega_d$$

$$X(t) = e^{-\zeta \omega_n t} \left( A_1 e^{i\omega_d t} + A_2 e^{-i\omega_d t} \right)$$

→ Using Euler's identity :

$$X(t) = e^{-\zeta \omega_n t} (B_1 \cos(\omega_d t) + B_2 \sin(\omega_d t))$$

→ Recalling The I.C's →  $X(0) = X_0$ ,  $\dot{X}(0) = \dot{X}_0$   
to find  $B_1$  and  $B_2$

①  $X_0 = B_1$  →  $B_1 = X_0$

②  $\dot{X}(t) = -\zeta \omega_n e^{-\zeta \omega_n t} (B_1 \cos(\omega_d t) + B_2 \sin(\omega_d t))$

هون اشتقيت معادلة  $X(t)$  ال عشوائية  
ال  $\dot{X}_0$  وأعرف  $B_2$  }  $+ e^{-\zeta \omega_n t} (-B_1 \omega_d \sin(\omega_d t) + B_2 \omega_d \cos(\omega_d t))$

$$\dot{X}_0 = V_0 = -\zeta \omega_n B_1 + B_2 \omega_d$$

$$V_0 = -\zeta \omega_n X_0 + B_2 \omega_d$$

$$B_2 = \frac{V_0 + \zeta \omega_n X_0}{\omega_d}$$

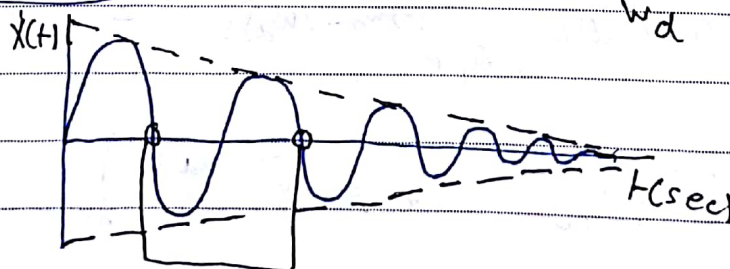
مع بالهارة  
ال عشوائية

$$\Rightarrow X(t) = e^{-\zeta \omega_n t} \left( X_0 \cos \omega_d t + \frac{V_0 + \zeta \omega_n X_0}{\omega_d} \sin(\omega_d t) \right)$$

هنا الجزء  
هو المسؤول  
عنا ال

Decay

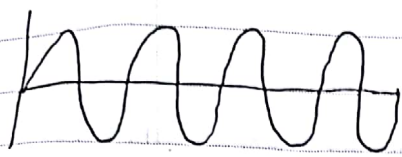
(exponential  
Decay)



$$T_d \text{ (damped period)} = \frac{2\pi}{\omega_d} = \frac{2\pi}{\omega_n \sqrt{1-\zeta^2}}$$

هنا تأثير ال Damping

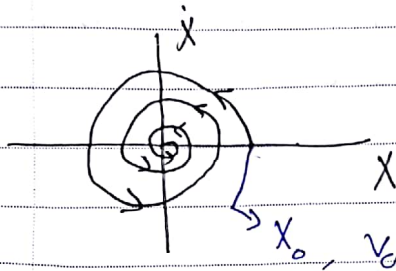




\* بدون دامپر فاني Friction Force ←

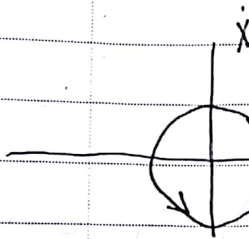
→ Energy is conserved

\* لو بدي ارسام على  $\dot{x}$  vs  $x$  (Phase Plane Plot)



→ Damping (No Energy Conservation)

مثلاً  $x_0, v_0 = 0$



→ No Dampin (Energy is conserved)

مثلاً  $x_0, v_0 = 0$

\* اذا كان في Decay يعني بروج للجهز ..

معانوا  $\zeta$  اعلى ،  $c$  اعلى

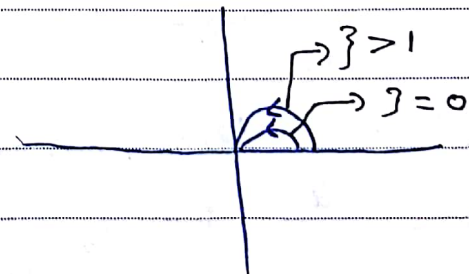
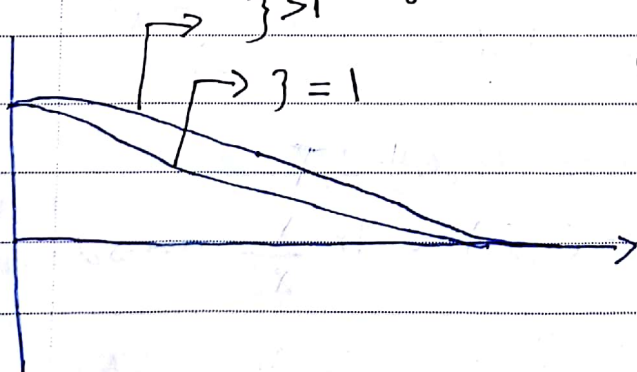
واذا خيل ثابت معانوا  $\zeta = 0, c = 0$  ..

Overdamped and

Critically damp case ( $\zeta = 1, \lambda_{1,2} = -\omega_n$ )

(No vibration)!

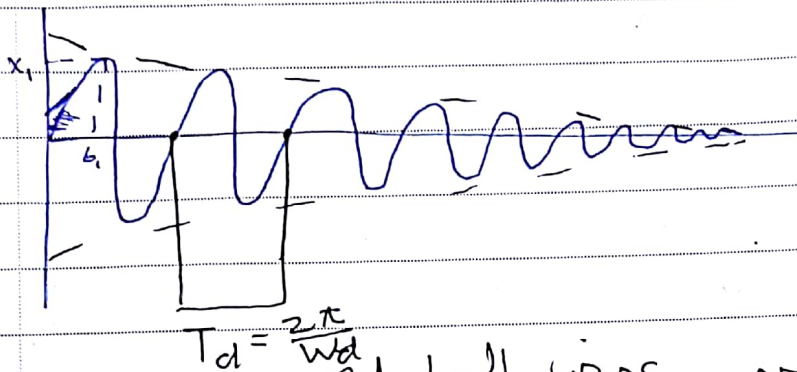
→ repeated roots



## \* Logarithmic Decrement:

→ Used to find experimentally the amount of damping in the system.

\* مثلاً لو كان في عندي Spring أفكر عليه بقوة معينة وعند اد  
وصي Stop Watch بقدر أحسب



عندي  $X_1$  و  $t_1$   $\xrightarrow{\text{باللحار}}$   $\sin$  بدلتا

$$X(t) = e^{-\beta \omega_n t} (A \cos(\omega_d t + \phi))$$

$$X_1 = e^{-\beta \omega_n t_1} (A \cos(\omega_d t_1 + \phi)) !$$

\* But  $t_2 = t_1 + T_d$

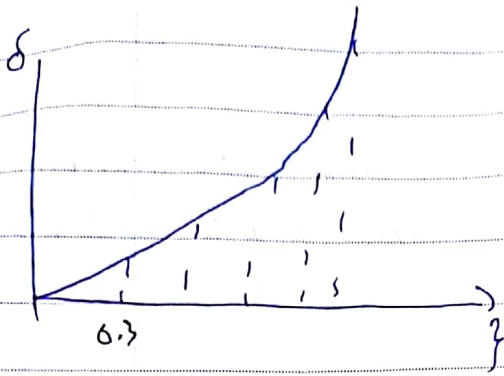
$$X_2 = e^{-\beta \omega_n t_2} (A \cos(\omega_d t_2 + \phi)) \xrightarrow{\frac{2\pi}{\omega_d} * \omega_d = 2\pi}$$

$$= e^{-\beta \omega_n t_1 - \beta \omega_n T_d} (A \cos(\omega_d(t_1) + 2\pi + \phi))$$

→ define New Parameter ( $\delta$ ) =  $\ln \frac{X_1}{X_2}$   $\xrightarrow{\text{logarithmic decrement}}$

$$\delta = \ln e^{\beta \omega_n T_d} = \beta \omega_n T_d = \frac{\beta \omega_n 2\pi}{\sqrt{1-\beta^2} \omega_n} \Rightarrow \delta = \ln \frac{X_1}{X_2} = \frac{2\pi\beta}{\sqrt{1-\beta^2}}$$





$$\delta = \frac{2\pi\zeta}{\sqrt{1-\zeta^2}}$$

→ For Lightly damped system  $\zeta \ll 1$

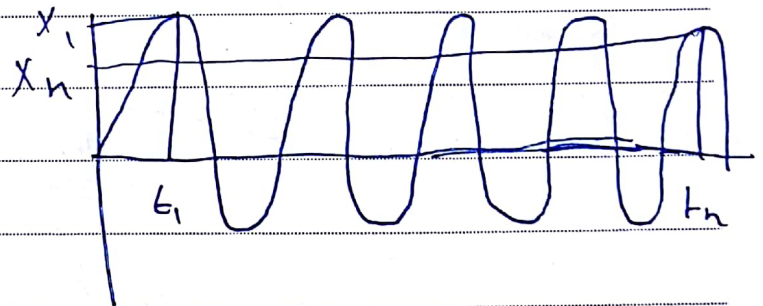
$$\ln \frac{x_1}{x_2} = \delta \cong 2\pi\zeta \Rightarrow \zeta = \frac{1}{2\pi} \ln \frac{x_1}{x_2}$$

→ For very Lightly Damped (بضعف قليل تأثير الـ Damping)

فـ  $x_1$  و  $x_2$  يـ  $n$  cycles  $L \ll n$  cycles  $n$  cycles

$$\Rightarrow x_1, x_n \quad ! \quad x_1 \text{ بـ } x_2 \text{ بـ } x_1$$

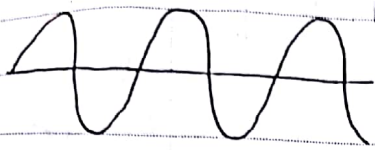
$$\Rightarrow t_n = t_1 + n T_d$$



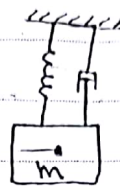
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Recall:

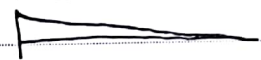


↓ x

Static equilibrium  
with  $x_0, \dot{x}_0$  as IC's

$$m\ddot{x} + Kx + c\dot{x} = 0 \quad \text{--- ①}$$

$$W_n = \sqrt{\frac{K}{m}}, \quad C_{cr} = \sqrt{4Km}, \quad \gamma = \frac{C}{C_{cr}}$$

Cases: ① Undamped ( $c=0, \gamma=0$ )② Underdamped ( $0 < \gamma < 1$ )③ Critically damped ( $\gamma=1$ )④ Overdamped ( $\gamma > 1$ )

$$\delta = \ln \frac{x_1}{x_2} \rightsquigarrow \delta = 2\pi\gamma \quad (\text{Lightly damped})$$

divide eq ① over m to get

$$\ddot{x} + \frac{c}{m} \dot{x} + \frac{K}{m} x = 0$$

$$\frac{c}{m} = 2\gamma W_n$$

$$\frac{K}{m} = W_n^2$$

$$\Rightarrow \ddot{x} + 2\gamma W_n \dot{x} + W_n^2 x = 0 \quad \text{انته انه معادله لا زلزال يكون}$$

Generalized Undamped S.D.F System

ex: if the system Equation of Motion is  $\ddot{q} + 3\dot{q} + 25q = 0$   
we can find  $W_n$  and  $\gamma$  !

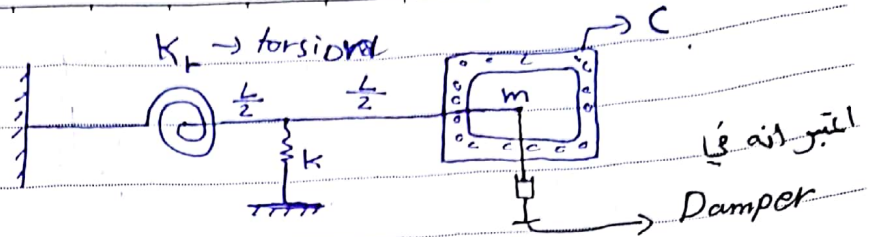
$$\begin{matrix} \uparrow & \uparrow \\ 2\gamma W_n & W_n^2 \end{matrix}$$

$$\rightarrow W_n^2 = 25 \rightarrow W_n = 5 \text{ rad/s} \quad 2\gamma W_n = 3 \rightarrow \gamma = \frac{3}{10}$$

(Underdamped!)



Example:



- Find: 1) Equation of motion, 2)  $C_{eq}$ , 3)  $K_{eq}$ , 4)  $m_{eq}$ , 5)  $\omega_n$ , 6) ?

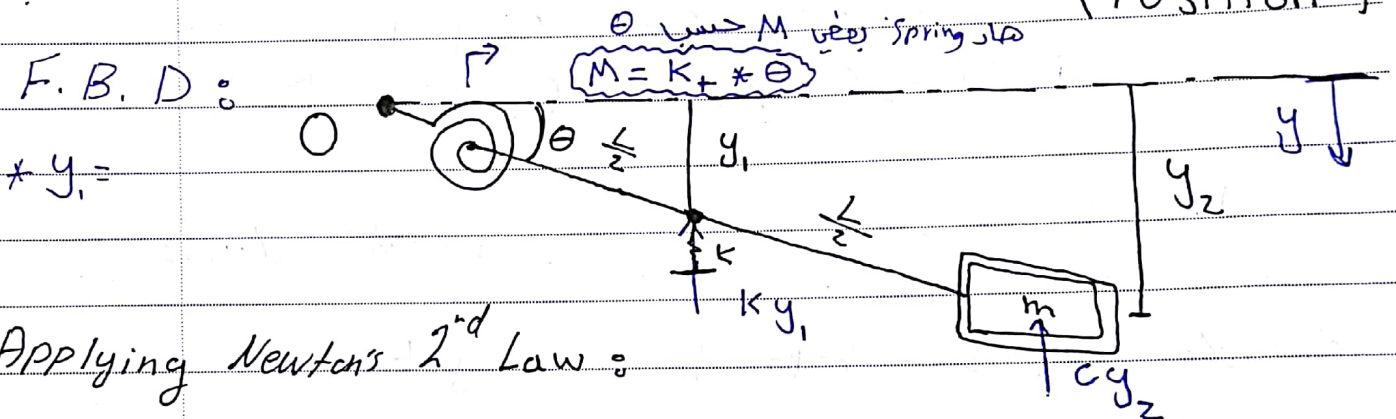
- Showing: 1) Applying Newton's 2<sup>nd</sup> Law

2) Free body diagram

3) The Coordinate System

\* the effect of  $mg$  is cancelled by  $K_{\delta_{st}}$  (Static equilibrium Position)

F.B.D:



Applying Newton's 2<sup>nd</sup> Law:

$$\sum M_o = J_o \ddot{\theta}$$

\*  $J_o$ : mass moment of inertia about O

$$ML^2 \ddot{\theta} = -K_t \theta - K y_1 \frac{L}{2} \cos \theta$$

$\rightarrow J_o = m L^2$  (mass of rod to the mass)

$$- C y_2 L \cos \theta$$

\* For small  $\theta$ ,  $\cos \theta = 1$  &  $\sin \theta = \theta$

$$\text{So } y_1 = \frac{L}{2} \theta, y_2 = L \theta$$

معادلة ال Moment كلها عبارة عن القوة \* المسافة يساوي الجهد

$$m L^2 \ddot{\theta} + K_t \theta + \frac{K L^2}{4} \theta + c L^2 \dot{\theta} = 0$$

No.

$$m_{eq} = mL^2$$

$$\underbrace{mL^2}_{m_{eq}} \ddot{\theta} + \underbrace{\left(K_t + \frac{KL^2}{4}\right)}_{K_{eq}} \theta + \underbrace{CL^2}_{C_{eq}} \dot{\theta} = 0$$

$$m_{eq} \ddot{\theta} + C_{eq} \dot{\theta} + K_{eq} \theta = 0$$

$$\Rightarrow \omega_n^2 = \frac{1}{m_{eq}} \left(K_t + \frac{KL^2}{4}\right)$$

$$\omega_n = \sqrt{\frac{K_t}{m_{eq}} + \frac{KL^2}{4m_{eq}}}$$

$$\rightarrow 2\zeta \omega_n = \frac{CL^2}{m_{eq}} \Rightarrow \zeta = \frac{CL^2}{2\omega_n m_{eq}}$$

$$\Rightarrow \ddot{\theta} + 2\zeta \omega_n \dot{\theta} + \omega_n^2 \theta = 0 \quad \begin{matrix} I'c'c = \theta(0) \\ \dot{\theta}(0) \end{matrix}$$

\* Notice that we can write the equation

$$\text{in terms of } y_2. \quad \theta = \frac{y_2}{L}, \quad \dot{\theta} = \frac{\dot{y}_2}{L}, \quad \ddot{\theta} = \frac{\ddot{y}_2}{L}$$

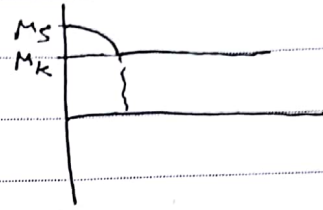
$$\ddot{y}_2 + 2\zeta \omega_n \dot{y}_2 + \omega_n^2 y_2 = 0$$

\* Notice that  $\omega_n$  &  $\zeta$  are independent from the Coordinate System, and will always have the Same Value.



Dry Friction Damping: "Columb damping"

$M_s \rightsquigarrow$  Static Coefficient of Friction



$M_k \rightsquigarrow$  Kinetic coefficient of Friction

$M$  depends on: 1) material properties of mating surfaces

2) Surface finish

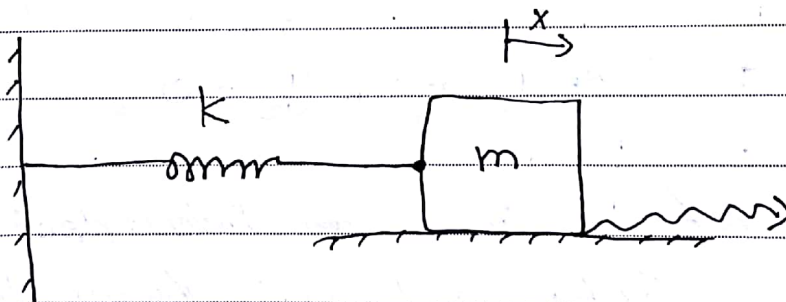
3) Lubricants

4) Temperature

$\rightarrow$  normal Force

$$F_{\text{friction}} = M * N$$

$\rightarrow$  irreversible, non-conservative, produce heat



Contact means there is

Friction Force

- Free Vibration :

- Dry Friction

$$\text{Period} = \frac{2\pi}{\omega_n} = T$$

→ Applying Newton's 2<sup>nd</sup> law

$$\text{at } 0 \leq t \leq \frac{T}{2} \text{ (half cycle)}$$

$$\sum F_x = m \ddot{x} \Rightarrow m \ddot{x} + kx = M * mg$$

$$\Rightarrow \text{General Solution: } X_g = X_{c.s} + X_{P.I}$$

$$\begin{cases} X(0) = X_0 \\ \dot{X}(0) = \text{Constant} \\ = \text{zero} \end{cases}$$

$$\rightarrow X_{c.s} = A \cos(\omega_n t) + B \sin(\omega_n t)$$

$$\rightarrow X_{P.I} = \text{Constant} \Rightarrow k_c = Mmg$$

$$C = \frac{Mmg}{k}$$

$$X_{g.s} = A \cos(\omega_n t) + B \sin(\omega_n t) + \frac{Mmg}{k}$$

$$\rightarrow X(0) = X_0 = A + \frac{Mmg}{k} \Rightarrow A = X_0 - \frac{Mmg}{k}$$

$$\rightarrow \dot{X}(0) = \dot{X}_0 = -A \sin(0) + B \cos(0) + 0 = B = \dot{X}_0 = \text{zero}$$

$$\Rightarrow X_{g.s}(t) = \left( X_0 - \frac{Mmg}{k} \right) \cos(\omega_n t) + \frac{Mmg}{k}$$

$$\Rightarrow \dot{X}_{g.s}(t) = -\omega_n * \left( X_0 - \frac{Mmg}{k} \right) \sin(\omega_n t) \quad * 0 \leq t \leq \frac{T}{2}$$



No.

→ Now we will find  $X(\frac{T}{\omega_n})$  &  $\dot{X}(\frac{T}{\omega_n})$  which will be our new IC for the 2<sup>nd</sup> half ( $\frac{T}{\omega_n} < t < \frac{2T}{\omega_n}$ )

$$\begin{aligned} \rightarrow X(\frac{T}{\omega_n}) &= (X_0 - \frac{Mmg}{K}) * (-1) + \frac{Mmg}{K} \\ &= -X_0 + 2 \frac{Mmg}{K} \end{aligned}$$

→  $\dot{X}(\frac{T}{\omega_n}) = \text{Zero!}$  ( $V = \text{zero} \rightarrow \text{Switch direction!}$ )

→ Apply Newton's 2<sup>nd</sup> law ( $\frac{T}{\omega_n} < t < \frac{2T}{\omega_n}$ )

$$\sum F = m \ddot{X} \rightarrow m \ddot{X} + KX = -Mmg$$

↳ negative sign!  
! على الاتجاه

$$X_{g.s} = X_{c.s} + X_{PI}$$

Same as before ←

$$X_{PI} = C$$

$$KC = -Mmg$$

$$C = \frac{-Mmg}{K}$$

$$\textcircled{a} \quad \left. \begin{aligned} t = \frac{T}{\omega_n}, \quad X &= -X_0 + 2 \frac{Mmg}{K} \\ \dot{X} &= 0 \end{aligned} \right\} \text{I.C.}$$

$$\rightarrow X(\frac{T}{\omega_n}) = -X_0 + 2 \frac{Mmg}{K} = -A + 0 - \frac{Mmg}{K}$$

$$A = X_0 - 2 \frac{Mmg}{K} - \frac{Mmg}{K}$$

$$A = X_0 - \frac{3 Mmg}{K}$$

$$\dot{x}(t) = -A \omega_n \sin(\omega_n t) + B \omega_n \cos(\omega_n t)$$

$$\dot{x}\left(\frac{\pi}{\omega_n}\right) = 0, \quad B = 0$$

$$\rightarrow X_{g.s} = \left(X_0 - \frac{3Mmg}{k}\right) \cos(\omega_n t) - \frac{Mmg}{k}$$

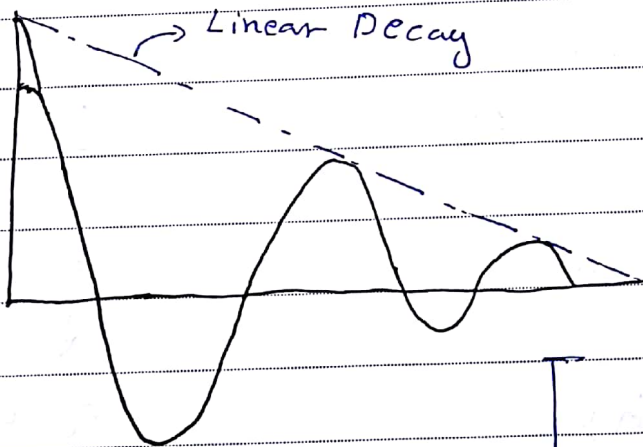
@ end of 2<sup>nd</sup> half cycle  $\left(\frac{2\pi}{\omega_n}\right)$

@  $\frac{\pi}{\omega_n} < t < \frac{2\pi}{\omega_n}$   
period

$$X\left(\frac{2\pi}{\omega_n}\right) = \left(X_0 - \frac{3Mmg}{k}\right) (1) - \frac{Mmg}{k}$$

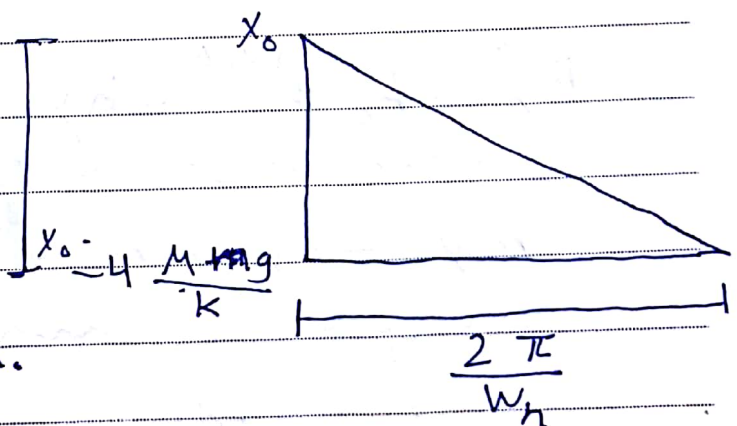
$$= X_0 - 4 \frac{Mmg}{k} !$$

× انه كل نصف دورة  $X$  يقل بقيمة ثابتة مقدارها  $\frac{2Mmg}{k}$  ، يعني  $\frac{2Mmg}{k}$  Linear Decay يكون !



× Dry friction  
doesn't effect  
the natural frequency

$$m = \frac{-4Mmg}{k} \div \left(\frac{2\pi}{\omega_n}\right) = \frac{\Delta Y}{\Delta X} \dots$$



# Chapter 3 : Forced Vibration of SDOF "Harmonic" Excitation

$$F(t) = F_0 \sin(\omega t) \quad \leftarrow \text{Amplitude} \quad \leftarrow \text{excitation frequency (rad/s)}$$

$$f = \frac{\omega}{2\pi}$$

$$= F_0 \cos(\omega t) \quad \leftarrow \text{Sin, cos, } e^{j\omega t}$$

$$= F_0 e^{j\omega t} \quad \leftarrow \text{Harmonic}$$

\* Undamped System ( $\begin{cases} c=0 \\ \gamma=0 \end{cases}$ ) :  $\text{بوفر من خلال}$

Actuator 1

example:

Rotating 2

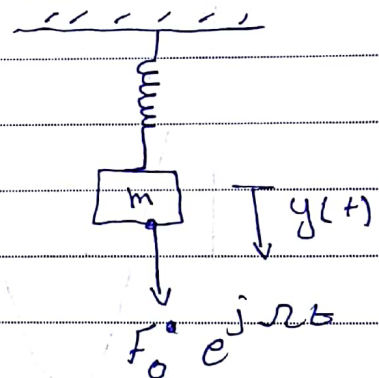
a spring with static position under Harmonic excitation: Unbalanced Machine

1 First we find the Equation of motion

→ Newton's 2<sup>nd</sup> law:

$$m\ddot{y} = F_0 e^{j\omega t} - Ky$$

$$m\ddot{y} + Ky = F_0 e^{j\omega t}$$



$y(0)$  &  $\dot{y}(0)$  are IC's

→ 2<sup>nd</sup> order O.P.E BUT non-homogeneous



as previous solution ..

$$y_{g.s} = y_{h.s} + y_{PI}$$

$$y_{c.s} = A \cos(\omega_n t) + B \sin(\omega_n t)$$

→ Linear System Output follows input ..

So we assume:

$$\left. \begin{aligned} y(t)_{PI} &= Y e^{j\omega t} \\ \dot{y}(t) &= Y j\omega e^{j\omega t} \\ \ddot{y}(t) &= -Y \omega^2 e^{j\omega t} \end{aligned} \right\}$$

$$m(-Y \omega^2 e^{j\omega t}) + k(Y e^{j\omega t}) = F_0 e^{j\omega t} \quad \text{Substitute in E.O.M}$$

$$(k - m \omega^2) Y e^{j\omega t} = F_0 e^{j\omega t}$$

$$Y = \frac{F_0}{k - m \omega^2}$$

$$x_{PI} = \frac{F_0}{k - m \omega^2} e^{j\omega t}$$

$$= \frac{F_0}{k - m \omega^2} \sin(\omega t)$$

$$= \frac{F_0}{k - m \omega^2} \cos(\omega t)$$

مردود  
من نفس  
التي  
قائم  
نفس  
التي

Tech  
Family

Let's Study

$$Y = \frac{F_0}{K - m \Omega^2}$$

→ divide by K

$$\omega_n^2 = \frac{k}{m}$$

$$Y = \frac{F_0 / K}{1 - \left(\frac{m}{K}\right) \Omega^2}$$

$$Y = \frac{Y_{static}}{1 - \frac{\Omega^2}{\omega_n^2}}$$

$$Y = \frac{Y_{static}}{1 - r^2}$$

1

$F_0 / K$ : Static deflection

$$y_{static} = \frac{F_0}{K}$$

2

$$r = \frac{\Omega}{\omega_n}$$

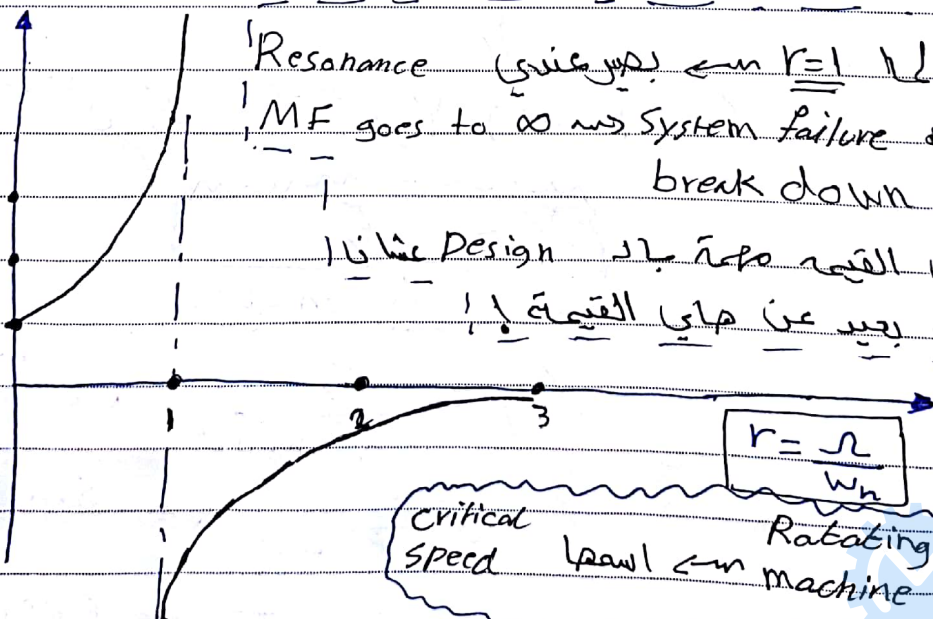
Frequency ratio

the ratio of  
external excitation  
frequency to the system  
natural frequency

$\frac{Y}{Y_{static}}$  → Magnification factor

→ draw  $r$  vs M.F :

$$\frac{Y}{Y_{static}}$$



$$\frac{Y}{Y_{st}} = \frac{1}{1 - r^2}$$

$$r=1 \Rightarrow \omega_n = \Omega$$

Resonance

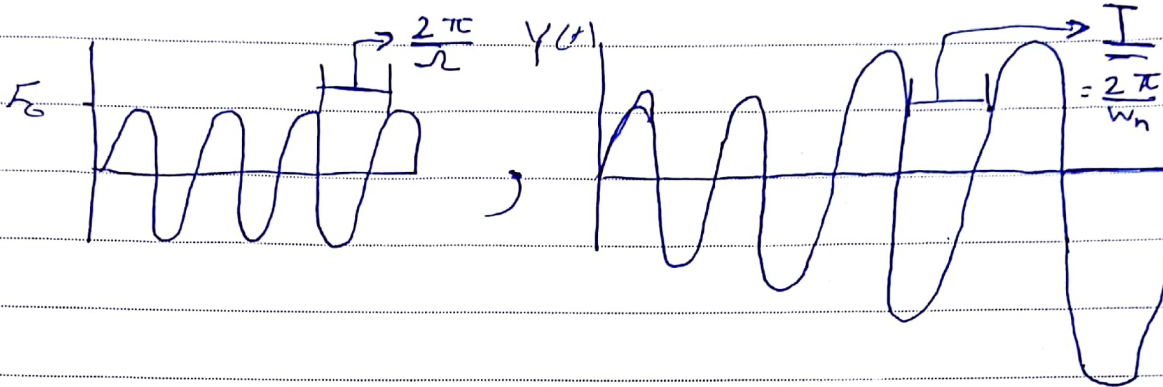
Critical Speed (السرعة الحرجة) ← Rotating machine

Provide 5 critical speed and How to Pass



No.

Resonance Occure when  $\omega_n = \omega$  (Same phase)



عند هيك بيلش كبر و بروج الى  $\infty$  !

23/10/2017

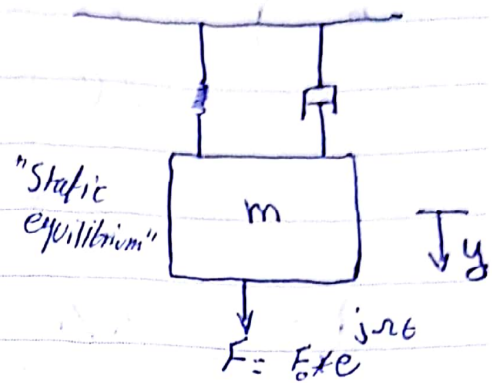
No.

For underdamped ( $0 < \zeta < 1$ )

Equation of motion:

$$m \ddot{y} + c \dot{y} + Ky = F_0 e^{j\omega t}$$

while  $y(0)$  &  $\dot{y}(0)$  are IC's



$$y_{g.s} = y_{c.s} + y_{PI}$$

\* For Linear System  $\rightarrow$  Output follows the input BUT with Lag!

For the Particular Solution  $\rightarrow$

$$y_{PI} = Y \cdot e^{j\omega t}$$

$$\dot{y}_{PI} = Y j\omega e^{j\omega t}$$

$$\ddot{y}_{PI} = -Y \omega^2 e^{j\omega t}$$

Substitute in EoM

$$\rightarrow -m Y \omega^2 e^{j\omega t} + c Y j\omega e^{j\omega t} + K Y e^{j\omega t} = F_0 e^{j\omega t}$$

$$\rightarrow Y = \frac{F_0}{K - m\omega^2 + cj\omega}$$

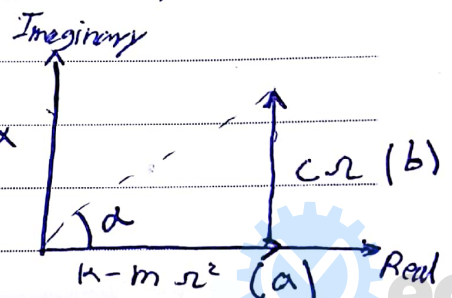
Complex Dynamic Stiffness

$$a + jb = \sqrt{a^2 + b^2} e^{j\alpha}$$

real      imaginary

$$\alpha = \tan^{-1}\left(\frac{b}{a}\right)$$

So  $\rightarrow K - m\omega^2 + c\omega j = \sqrt{(K - m\omega^2)^2 + (c\omega)^2} e^{j\alpha}$



and  $\alpha = \tan^{-1}\left(\frac{c\omega}{K - m\omega^2}\right)$

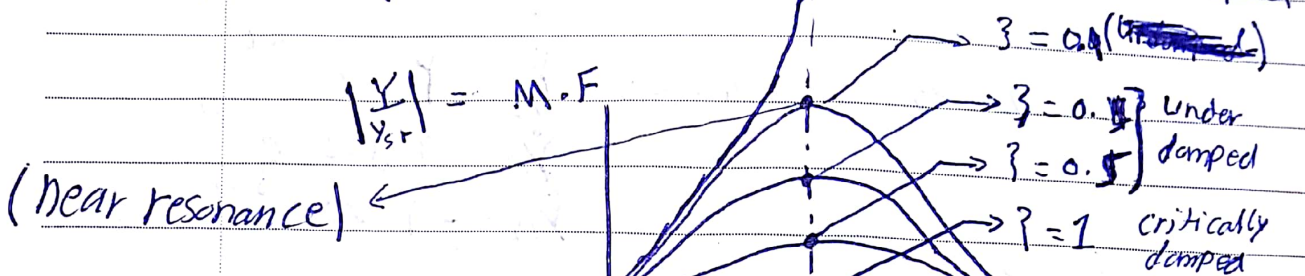
فارجز انصفي لاننا بس محتاج القيمة  $\sqrt{(K - m\omega^2)^2 + (c\omega)^2}$



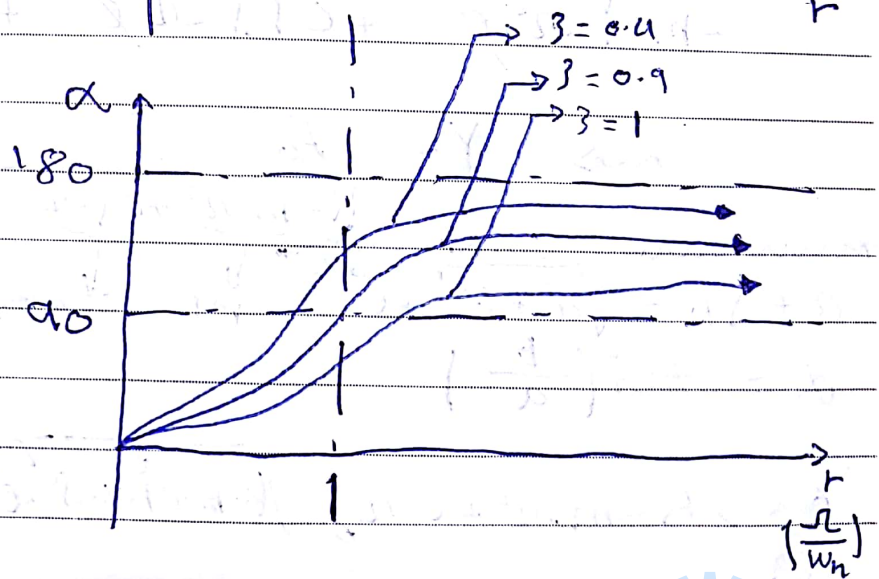
$$Y = \frac{F_0 e^{j\omega t}}{\sqrt{(k - m\omega^2)^2 + (c\omega)^2}} \rightarrow \text{divide by } (k)$$

$$Y = \frac{F_0/k}{\sqrt{(1 - \frac{\omega^2}{\omega_n^2})^2 + (2\zeta r)^2}} \rightarrow y_{\text{static!}} \text{ and } \boxed{r = \frac{\omega}{\omega_n}}$$

$$\frac{Y}{y_{\text{st}}} = \frac{1}{\sqrt{(1-r^2)^2 + (2\zeta r)^2}} = M.F$$



ازاى  $\alpha$   $\rightarrow$   $\alpha$   $\times$   
 Phase Shift  $\rightarrow$   
 input & output  $\omega$



Plot the Curve :

$r = (0.5 - 5)$ , increment (0.1)  
for  $\zeta = 0.1, 0.8, 1, 1.2$

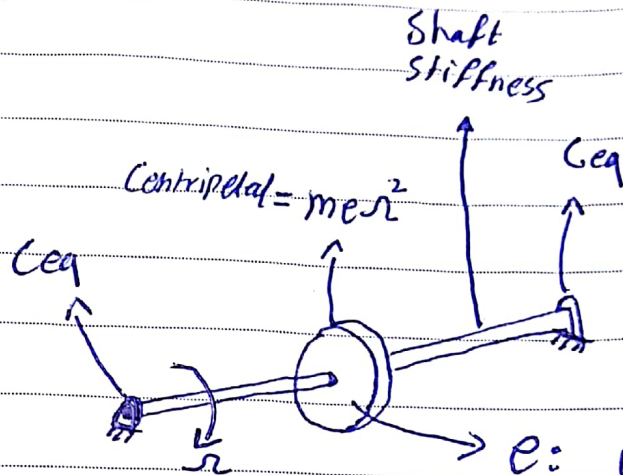
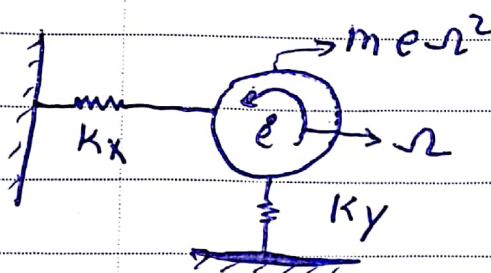
X-axis  $\rightarrow r$

Y-axis  $\rightarrow \frac{y}{y_{st}}$

$$y_{c.s} = e^{-\zeta \omega_n t} (A \cos(\omega_d t) + B \sin(\omega_d t))$$

$$y_{P.I} = \frac{F_0/k}{\sqrt{(1-r^2)^2 + (2\zeta r)^2}} * e^{-\zeta \omega_n t}$$

\* Jeffcot rotor :



$e$ : eccentricity  
" difference between  
mass center and  
the geometrically  
rotation center

Home work: [1] Plot (the above)

[2] Find the equation of motion  
for Jeffcot rotor | مراجعة بالكاتب

Family



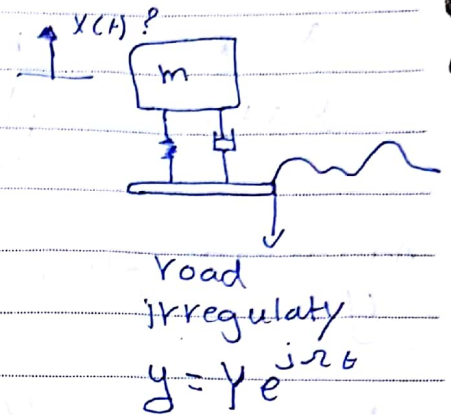
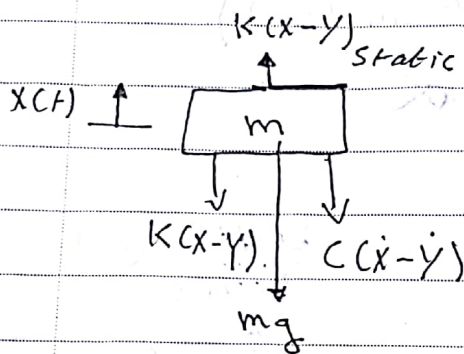
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# Response of SDOF System under Base Harmonic excitation

The equation of motion

Free body diagram assuming  $x > y$



\* Applying Newton's 2<sup>nd</sup> law:

$$\sum F_x = m \ddot{x} = -K(x-y) - C(\dot{x}-\dot{y})$$

$$\Rightarrow m \ddot{x} + C \dot{x} + Kx = C \dot{y} + Ky$$

\* as  $y(t)$  is known displacement

$x, \dot{x}$  @ Zero are our Initial Conditions

$$y = Y e^{j\omega t}, \quad \dot{y} = Y j \omega e^{j\omega t}, \quad \ddot{y} = -Y \omega^2 e^{j\omega t}$$

Substitute in the original Equation:

$$m \ddot{x} + C \dot{x} + Kx = C(Y j \omega e^{j\omega t}) + K(Y e^{j\omega t})$$

So we have Linear System Output that is following the input.

$$X_{g.s} = \overset{\text{homogeneous}}{X_{c.s}} + X_{PI}$$

For the particular Integral ( $X_{PI}$ )

$$x = X e^{j\omega t} \rightarrow \dot{x} = j\omega X e^{j\omega t} \rightarrow \ddot{x} = -\omega^2 X e^{j\omega t}$$

Substitute in the original Equation:

$$(k - m\omega^2) X e^{j\omega t} + j c \omega X e^{j\omega t} = (k + j c \omega) Y e^{j\omega t}$$

Output  $\leftarrow$

$$\frac{X}{Y} = \frac{k + j c \omega}{(k - m\omega^2) + j c \omega}$$

input  $\leftarrow$

dimensionless and,  $k + j c \omega = \sqrt{k^2 + c^2 \omega^2} e^{j\phi}$

Displacement Transmissibility Ratio

$$\phi = \tan^{-1} \left( \frac{c \omega}{k} \right)$$

$$\text{and } (k - m\omega^2) + j c \omega = \sqrt{(k - m\omega^2)^2 + c^2 \omega^2} e^{j\beta}$$

\*\*\*

$$\beta = \tan^{-1} \left( \frac{c \omega}{k - m\omega^2} \right)$$

$$\frac{X}{Y} = \frac{\sqrt{1 + (2\zeta r)^2}}{\sqrt{(1 - r^2)^2 + (2\zeta r)^2}}$$

$$\sqrt{(1 - r^2)^2 + (2\zeta r)^2}$$

divide by k to get

$$r = \frac{\omega}{\omega_n}, \zeta = \frac{c}{c_{cr}}$$



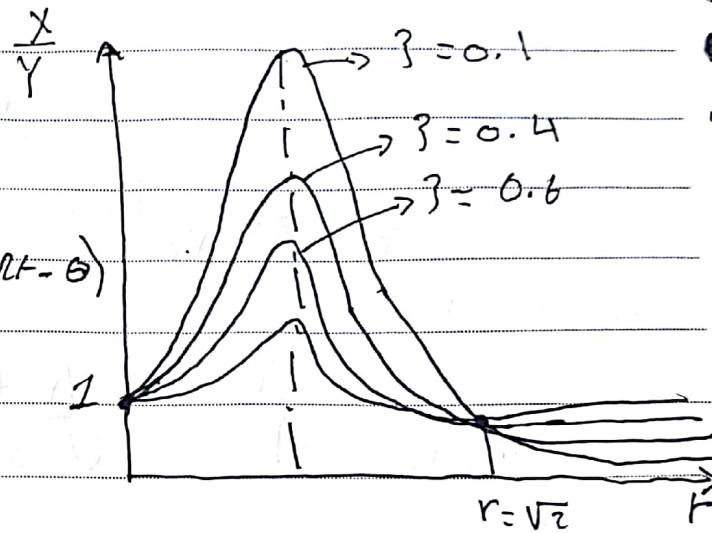
\* من المعادلة لأي إذا ما كان غيري Damping  $C=0$

$$\frac{X}{Y} = \frac{1}{1-r^2} \quad \text{بتحليل المعادلة}$$

\* to find The Maximum Value  $\rightarrow$  derivative  $\left( \frac{d(\frac{X}{Y})}{dr} \right)$

$$X_p(t) = \frac{Y \sqrt{1 + (2\zeta r)^2}}{\sqrt{(1-r^2)^2 + (2\zeta r)^2}} e^{j(\omega t - \theta)}$$

$$X_p(t) = \frac{Y \sqrt{1 + (2\zeta r)^2}}{\sqrt{(1-r^2)^2 + (2\zeta r)^2}} \times \sin(\omega t - \theta)$$

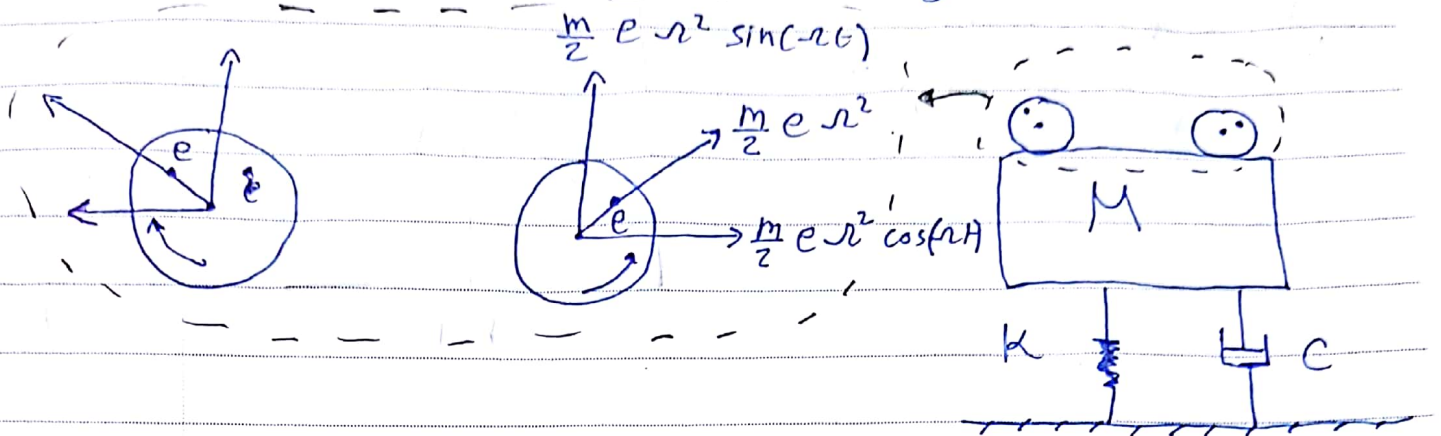


$$X_{g.s} = e^{-\zeta \omega_n t} (A \cos \omega_d t + B \sin \omega_d t) + P'$$

$$+ \frac{Y \sqrt{1 + (2\zeta r)^2}}{\sqrt{(1-r^2)^2 + (2\zeta r)^2}} \times \sin[\omega t - \theta]$$

$\rightarrow$  Apply The I.C'S to find A and B.

# Response of SDOF due to rotating unbalance :

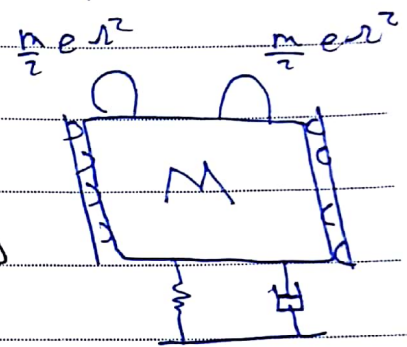


الـ  $e$  هي الـ Eccentricity وهي بعد الـ Center of mass

عن الـ Center of rotation

وهي السبب الرئيسي لـ Vibration

وأيضا إذا كان نفس بعض فالحالة للعين واليسار بلغة بعض!



⇒ Replace  $F_0 = m e \Omega^2$

$$X(t) = X e^{j\Omega t}, \quad F(t) = m e \Omega^2 e^{j\Omega t}$$

$$X = \frac{F_0}{\sqrt{(k - M\Omega^2)^2 + (c\Omega)^2}} \quad \text{if we divide by } k$$

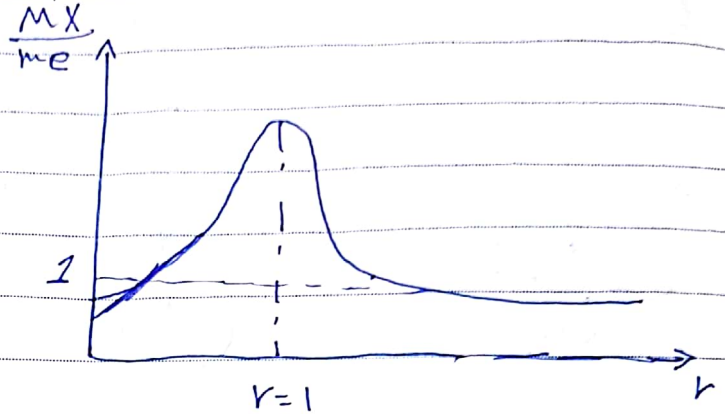
$$\text{and } r = \Omega, \quad \beta = \frac{c}{c_{cr}}, \quad k = M\omega_n^2$$

$$\frac{\Omega^2}{\omega_n^2} = r^2 = M$$

$$\frac{M X}{m e} = \frac{r^2}{\sqrt{(1 - r^2)^2 + (2\beta r)^2}}$$



$$\frac{x}{y} = \frac{\sqrt{1 + (23r)}}{\sqrt{(1-r^2)^2 + (23r)^2}}$$



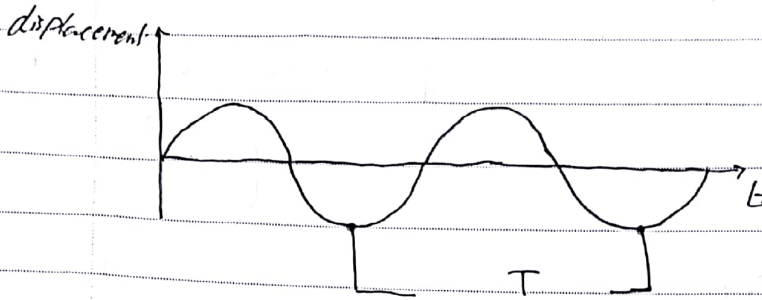
End of  
first exam  
material

1/11/2017

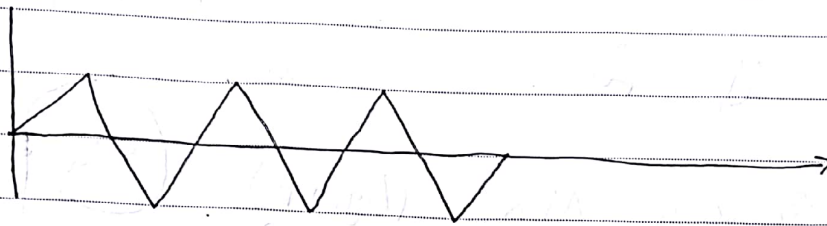
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## Revision lecture

\* **Vibration:** Dynamic Motion that repeats it self.



→ period (seconds) (time) to Complete one Cycle



**Frequency (f):** No. of cycle Completed in one Second (CPS)  
(Hz)

Mathimatically:  $\omega = 2\pi f = \frac{2\pi}{T}$  (rad/s)

\*  $f = \text{rps} = \frac{\text{rpm}}{60}$

\* **Classification**

- Free Vibration → Mathematical Model by Physical rules (Newton)
- Forced Vibration

No. of independent Position Co-ordinate

Applied to each

DoF

→ F.B.D

with Assumption

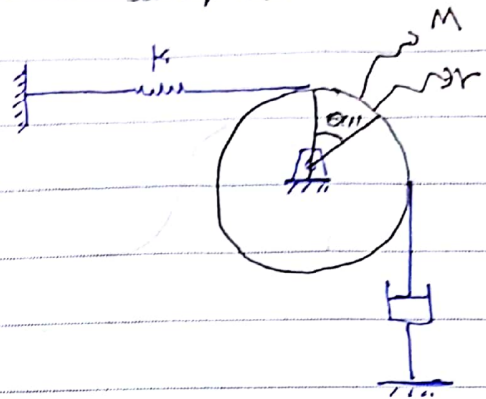


For Free vibration:  $x(t)$ ,  $y(t)$ ,  $\theta(t)$ ,  $z(t)$

Classification: - Undamped - Under damped  
- Critically damped - Over damped

Assumptions:

- ① Small motions
- ② Linear Spring
- ③ Viscous damper

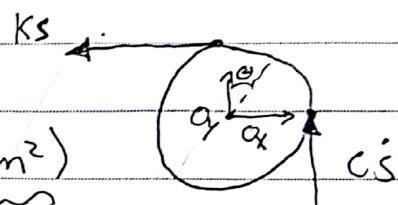


applying Newton's 2<sup>nd</sup> law:

\*The F.B.D is drawn with deflections

$$\sum M_o = J_o \ddot{\theta}$$

$J_o \rightarrow$  For disk  $\frac{1}{2} M R^2$  ( $\text{kg} \cdot \text{m}^2$ )



$$\frac{1}{2} M R^2 \ddot{\theta} = -K s r - C \dot{s} r$$

replace  $s$  by  $\theta$

$$\begin{aligned} s &= r \theta \\ \dot{s} &= r \dot{\theta} \end{aligned}$$

$$\Rightarrow \frac{1}{2} M R^2 \ddot{\theta} + K r^2 \theta + C r^2 \dot{\theta} = 0$$

$$M \ddot{\theta} + 2C \dot{\theta} + 2K \theta = 0 \rightarrow \text{divide by } M$$

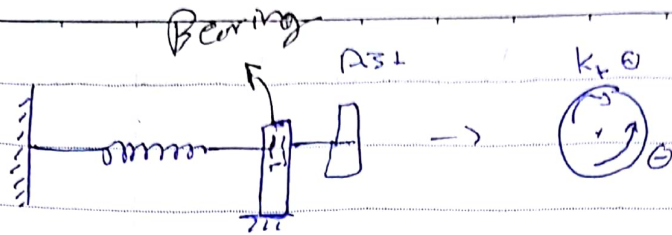
23 Nov

$$\ddot{\theta} + \frac{2C}{M} \dot{\theta} + \frac{2K}{M} \theta = 0$$

$$\omega_n = \sqrt{\frac{2K}{M}} \text{ (rad/s)}$$

$$\zeta = \frac{C}{M \omega_n}$$

No.



$$\sum M_o = J_o \ddot{\theta} + k_t \theta$$

$$J_o \ddot{\theta} + k_t \theta = 0$$

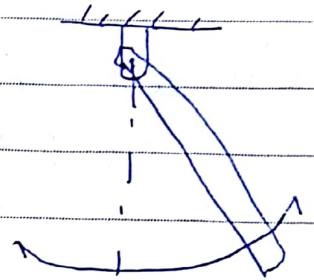
$$\ddot{\theta} + \frac{k_t}{J_o} \theta = 0$$

$$\omega_n^2 = \frac{k}{J_o}$$

$$\omega_n = \sqrt{\frac{k}{J_o}}$$

For Compound Pendulum (like Normal Pendulum)

$$\omega_n = \sqrt{\frac{g}{L}}$$

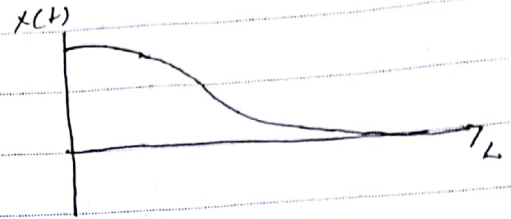




نقطة استقار المعادلات لاخر حالتين  
Critically damped ( $\beta=1, C=C_r$ )

$$\lambda_{1,2} = \left( -\beta \pm \sqrt{\beta^2 - 1} \right) \omega_n$$

$$= -\omega_n$$



$$\leadsto x(t) = C_1 e^{-\omega_n t} + C_2 t e^{-\omega_n t}$$

$$= e^{-\omega_n t} (C_1 + C_2 t)$$

if  $x(0) = x_0, \dot{x}(0) = \dot{x}_0 \leadsto C_1 = x_0, C_2 = \dot{x}_0 + x_0 \omega_n$

$$x(t) = (x_0 + (\dot{x}_0 + x_0 \omega_n) t) e^{-\omega_n t}$$

Over damped  $\sqrt{\beta^2 - 1} \leadsto$  Positive

$$\lambda_1 = \left( -\beta + \sqrt{\beta^2 - 1} \right) \omega_n$$

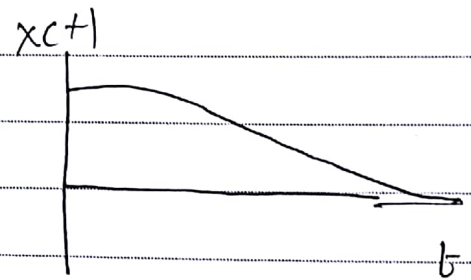
$$\lambda_2 = \left( -\beta - \sqrt{\beta^2 - 1} \right) \omega_n$$

Both will always be negative

$$x(t) = C_1 e^{(-\beta + \sqrt{\beta^2 - 1}) \omega_n t} + C_2 e^{(-\beta - \sqrt{\beta^2 - 1}) \omega_n t}$$

For  $x(0) = x_0, \dot{x}(0) = \dot{x}_0 \leadsto$

$$C_1 = \frac{x_0 \omega_n (\beta + \sqrt{\beta^2 - 1}) + \dot{x}_0}{2 \omega_n \sqrt{\beta^2 - 1}}$$



$$C_2 = \frac{-x_0 \omega_n (\beta - \sqrt{\beta^2 - 1}) - \dot{x}_0}{2 \omega_n \sqrt{\beta^2 - 1}}$$

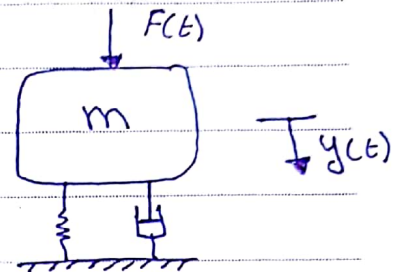
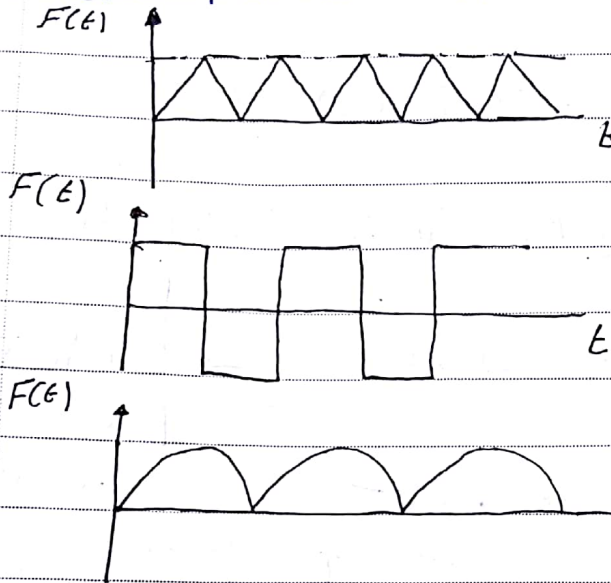
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# Vibration

## Chapter 4: Response under General loading

### General periodic force

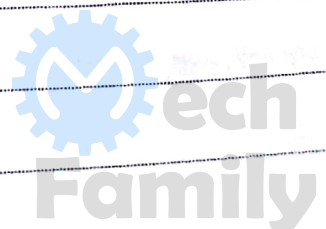


\* Any periodic function Can be represented as a series Summation using Fourier Series.

$$F(t) = \frac{a_0}{2} + \sum_{i=1}^{\infty} a_i \cos(i\omega t) + b_i \sin(i\omega t)$$

where  $\omega = \frac{2\pi}{T} = 2\pi f$

Fundamental  
Frequency





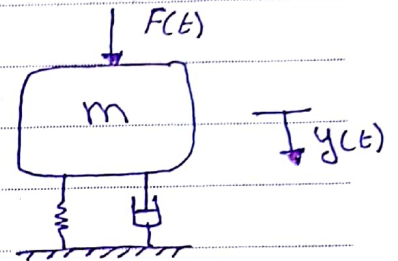
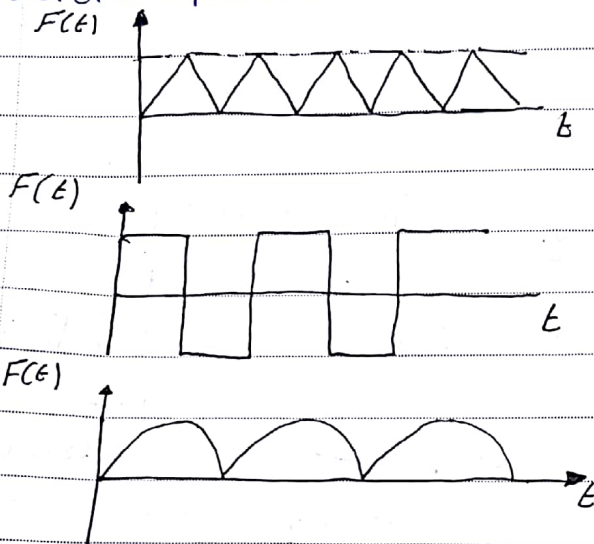
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# Vibration

## Chapter 4: Response under General loading

### □ General periodic force



\* Any periodic function can be represented as a series summation using Fourier series.

$$F(t) = \frac{a_0}{2} + \sum_{i=1}^{\infty} a_i \cos(i\omega t) + b_i \sin(i\omega t)$$

where  $\omega = \frac{2\pi}{T} = 2\pi f$

Fundamental frequency

Multiply both sides of the equation by  $\cos(i\omega t)$  and integrate over one period

$$\int_0^T F(t) \cos(i\omega t) dt = \int_0^T \frac{a_0}{2} \cos(i\omega t) dt$$

$$+ \sum \int_0^T a_i \cos^2(i\omega t) dt$$

$0 \rightarrow \infty$

$$+ \sum \int_0^T b_i \sin(i\omega t) \cos(i\omega t) dt$$

$$* a_i = \frac{2}{T} \int_0^T F(t) \cos(i\omega t) dt$$

$$* b_i = \frac{2}{T} \int_0^T F(t) \sin(i\omega t) dt$$

The Fourier Coefficients:

$$+ m \ddot{y} + c \dot{y} + k y = F(t)$$

$$\Rightarrow m \ddot{y} + c \dot{y} + k y = \frac{a_0}{2} + \sum_{i=1}^{\infty} a_i \cos(i\omega t) + \sum_{i=1}^{\infty} b_i \sin(i\omega t)$$

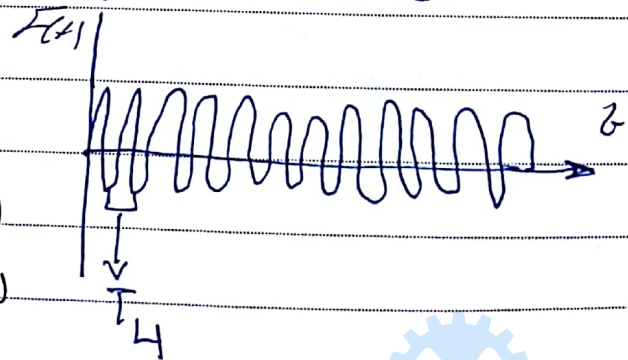
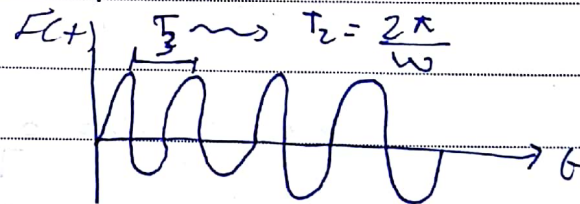
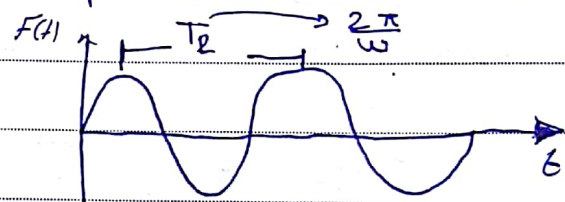
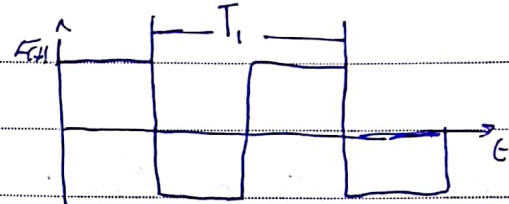
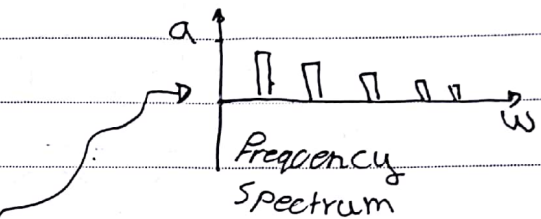
after solving the O.D.E for the given forcing function  $y_{gs} = y_{c.s} + y_{PI}$

Since we are dealing with linear system the method of superposition applies, then:

$$m \ddot{y}_i + c \dot{y}_i + k y_i = a_i \cos(i\omega t)$$

$$m \ddot{y}_i + c \dot{y}_i + k y_i = b_i \sin(i\omega t)$$

$$m \ddot{y}_i + c \dot{y}_i + k y_i = \frac{a_0}{2}$$



See next Page for the General Eq.



\* Harmonic motion:

$$y(t) = \sum_{i=1}^{\infty} y_i(t), \quad \text{for all the harmonics.}$$

the particular Integral Solution is:

$$y_{PI} = \sum_{i=0}^{\infty} \frac{i A_i / k \cos(i\omega t + \phi_i)}{\sqrt{(1 - (ir)^2)^2 + (2\beta ir)^2}} + \frac{i B_i / k \sin(i\omega t + \phi_i)}{\sqrt{(1 - (ir)^2)^2 + (2\beta ir)^2}}$$

\* Resonance:

$$\left. \begin{array}{l} \omega = \omega_n \Rightarrow r = 1 \\ i\omega = \omega_n \Rightarrow r = i \end{array} \right\} \rightarrow 2 \text{ points!}$$

→ Possibilities of resonance increase!

the general equation for previous page

$$y_i(t) = \frac{a_i / k}{\sqrt{(1 - (ir)^2)^2 + (2\beta ir)^2}}, \cos(i\omega t - \phi)$$

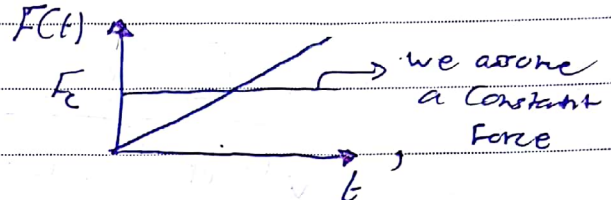
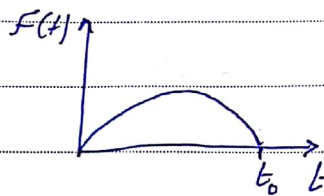
So if the General loading is:

1 Periodic  $\rightarrow$  Fourier analysis  $\rightarrow$  Cos & Sine  
exponential Form

2 Non-Periodic input:

- doesn't repeat it self, act for sometime and disappears.

example:

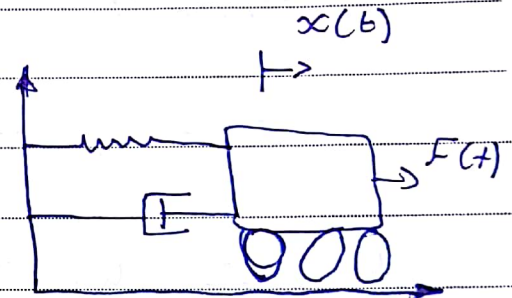


So for this system:

E.O.M:

$$m\ddot{x} + c\dot{x} + kx = F_c$$

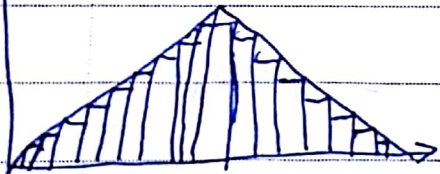
بدل از  $F(t)$



2<sup>nd</sup> order O.D.E!

مثال از لرزان عینی های ال Force

و قسمتی از مجموع Constant Force..



و عشان ال Vibration بفرض طبق  
علیه ال (Super position)

ال Output بساوی مجموع ال Output ال input



So we get : ① Response to each Pulse  
 ② Summation  
 ③ Duhamel's Integral (Convolution)

\* Alternative methods: ① Laplace transformation  
 ② Numerical integration

\* What actually happens in Laplace domain we freeze the time and study the behaviour in  $s$ -domain,  $s$  is  $(i\omega)$ .

\* it's more like conversion

$$\mathcal{L}\{f(t)\} = \int_0^{\infty} f(t) * e^{-st} dt = F(s)$$

Time-domain
 $s$ -domain

↓
↓

\* Laplace Function Conditions:

\* anything physical can be expressed in Laplace!

\* it Converts From differential to polynomial expressions.

\* Valid method for Solving Linear System differential equation

\* it takes care of the initial condition automatically

\* to go to time domain we do inverse Laplace transformation.

~~~~~  
Check Laplace transformation table for solving

Section 4.7  $\rightarrow$  Very important!

~~~~~  
The SDOF System:

$$m \ddot{x} + c \dot{x} + k x = F(t)$$

taking the Laplace

$$\mathcal{L}(m \ddot{x} + c \dot{x} + k x) = \mathcal{L}(F(t))$$

\* For  $I'c = \text{zero}$





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Recall  $\rightarrow X(t) = \int_0^{\infty} x(t) e^{-st} dt$  Linear ODE

Our equation is  $m \frac{d^2 x}{dt^2} + c \frac{dx}{dt} + kx = F(t)$

$$L(x(t)) = X(s)$$

For  $\dot{x} \rightarrow L\left(\frac{dx}{dt}\right) = \int_0^{\infty} \frac{dx}{dt} e^{-st} dt$   $\rightarrow$  integration by Parts

$$u = e^{-st} \rightarrow du = -s e^{-st}$$

$$dv = \frac{dx}{dt} \rightarrow v = x(t)$$

$$L\left(\frac{dx}{dt}\right) = s X(s) - x(0)$$

$$L\left(\frac{d^2 x}{dt^2}\right) = s^2 X(s) - s x(0) - \dot{x}(0)$$

\* From Laplace Transform table  $\rightarrow L(c) = \frac{c}{s}$   $\rightarrow$  Step input  
 $L(ct) = \frac{c}{s^2}$   $\rightarrow$  Ramp input

$$L(m \ddot{x} + c \dot{x} + kx) = L(F(t))$$

$$m(s^2 X(s) - s x(0) - \dot{x}(0)) + c(s X(s) - x(0)) + k(X(s)) = F(s)$$

$$X(s) (ms^2 + cs + k) = F(s) + x(0)(ms + c) + m \dot{x}(0)$$

Laplace take care of Initial Conditions automatically!

ech  
Family

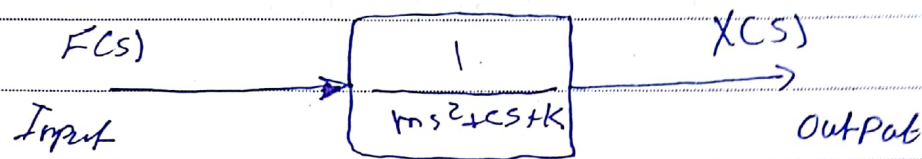
For Zero I.C.s  $\rightarrow X(0) = 0, \dot{X}(0) = 0$

$$X(s) = \frac{F(s)}{ms^2 + cs + k}$$

Transfer Function  $\rightarrow \frac{\text{Output}}{\text{Input}}$

$$= \frac{X(s)}{F(s)} = \frac{1}{ms^2 + cs + k}$$

drawing the Block diagram:



$X(s)$

after solving for the Output  $\uparrow$  back to time domain (Laplace Transform ~~is~~ (inverse))

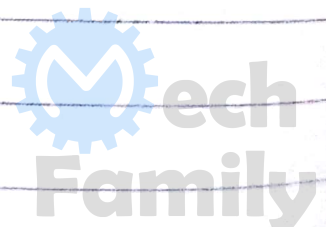
$$x(t) = \mathcal{L}^{-1}(X(s))$$

\* Final and Initial Value Theorem. (Steady State and

$\rightarrow$  this involves complex integral

Usually we use tables and for linear system

$\rightarrow$  any problem can be solved by Partial Fraction Expansion





Partial Fraction Example:

$$X(s) = \frac{s}{s^2 + 3s + 2}$$

حار الشكك الى Laplace inverse  
فجعل partial fraction expansion

$$X(s) = \frac{s}{s^2 + 3s + 2} = \frac{a}{(s+1)} + \frac{b}{(s+2)}$$

←  $= (s+1)(s+2)$

وحد مقاماتنا  
عشان البسط  
يساوي البسط

$$s = a(s+2) + b(s+1)$$

عوض  $-2, -1$   
عشان تووجد  
قيم  $a$  &  $b$

$s = -1$

$$-1 = a + 0$$

$$\leadsto a = -1$$

$$s = -2$$

$$-2 = 0 + -b$$

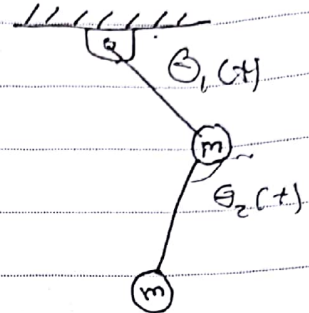
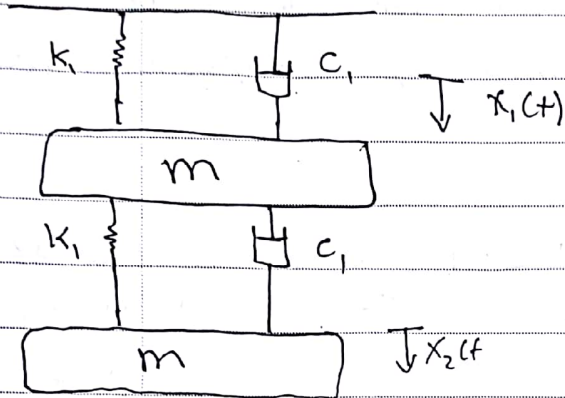
$$b = 2$$

$$\text{So } \leadsto \mathcal{L}^{-1} \left( \frac{-1}{(s+1)} + \frac{2}{s+2} \right) = -e^{-t} + 2e^{-2t} !$$

"Solved easily"

# Ch : Vibration of 2 DOF systems

## Examples



Position Coordinates needed

→ The minimum number ↑ to describe the motion is 2.

→ the 2 Coordinates are Completely independent from each other, can't be related in an equation to convert it into a SDOF.

\*For Free Vibration we are after "Natural Frequencies" and "modes" of Free Vibration

↓  
(العلاقة بين المركبات مع بعض)  
أو عكس بعض

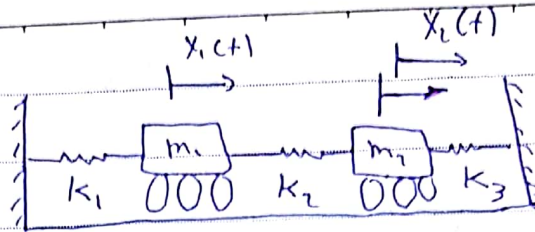
Steps:

- 1 First we should (idealize) our system "Choose the Coordinates"
- 2 apply physical laws to develop the E.O.M
- 3 Perform to the resulting equations

→ Natural Frequency and modes

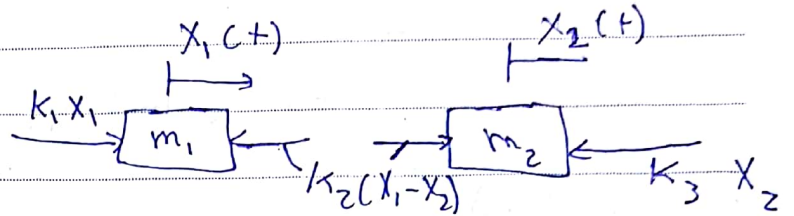


Example:

Apply Newton's 2<sup>nd</sup> law for each of the mass at a time

F.B.D

$$\bullet x_1(t) > x_2(t)$$

\* For  $m_1$ 

$$\sum F_{x_1} = m_1 \ddot{x}_1 = -k_1 x_1 - k_2 (x_1 - x_2)$$

$$\Downarrow$$

$$m_1 \ddot{x}_1 + (k_1 + k_2) x_1 - k_2 x_2 = 0 \quad \text{--- (1)}$$

\* For  $m_2$ :

$$\sum F_{x_2} = m_2 \ddot{x}_2 = k_2 (x_1 - x_2) - k_3 x_2$$

$$\Downarrow$$

$$m_2 \ddot{x}_2 = + (k_2 + k_3) x_2 - k_2 x_1 \quad \text{--- (2)}$$

These 2 equations

Should be used together

to be Solved

Re-arrange in matrix form Stiffness matrix

$$\begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} k_1+k_2 & -k_2 \\ -k_2 & k_2+k_3 \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

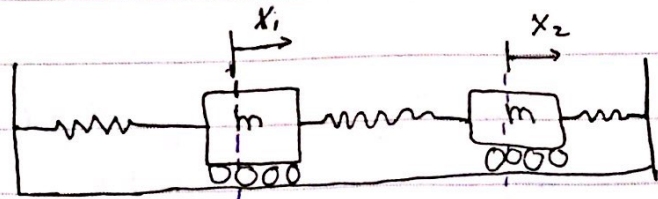
جدول بقیہ کے مطابق پڑھ لیں  
یہی نفسی حالتیں

mass matrix  
or  
Inertia matrix



## 2 DoF Systems

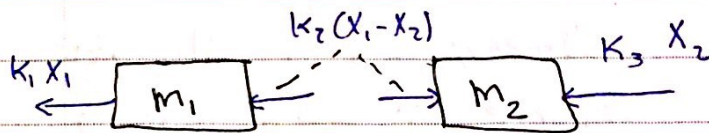
1- undamped



\* Free vibration (initial displacement)



\* Free body diagram

Assumption  $x_1 > x_2$ (بفرض الـ Spring اللي بين الماسين  $x_1 > x_2$ )Applying Newton's 2<sup>nd</sup> law :

$$(m_1) \quad m_1 \ddot{x}_1 = -k_1 x_1 - k_2 (x_1 - x_2)$$

$$\rightarrow m_1 \ddot{x}_1 + k_1 x_1 + k_2 (x_1 - x_2) = 0$$

$$(m_2) \quad m_2 \ddot{x}_2 = +k_2 (x_1 - x_2) - k_3 x_2$$

$$\rightarrow m_2 \ddot{x}_2 - k_2 (x_1 - x_2) + k_3 x_2 = 0$$

بشكل مatrix

$$\begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 + k_3 \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

\* In general, any two degree of freedom system

$$\begin{bmatrix} m_{ii} & m_{ij} \\ m_{ji} & m_{jj} \end{bmatrix} \begin{bmatrix} \ddot{x}_i \\ \ddot{x}_j \end{bmatrix} + \begin{bmatrix} k_{ii} & k_{ij} \\ k_{ji} & k_{jj} \end{bmatrix} \begin{bmatrix} x_i \\ x_j \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

if  $\left. \begin{array}{l} m_{ij} = m_{ji} \\ \& \\ k_{ij} = k_{ji} \end{array} \right\}$  Symmetric  $\rightarrow$  Positive Definite

يعني ال Determinate  $\neq 0$   
وبقدر أوجد ال Inverse

if  $m_{ij} = 0 \rightarrow$  Dynamically uncoupled

$m_{ij} \neq 0 \rightarrow$  Dynamically Coupled

\* Coupled  $\rightarrow$  معناه انه الحركتين مربوطتين ببعض  
يعني في علاقة بينهم !

if  $k_{ij} = 0 \rightarrow$  Staticaly uncoupled

$k_{ij} \neq 0 \rightarrow$  Statically Coupled

\* For the previous example \*\*\*

it is Dynamically uncoupled and  
Statically Coupled



## Free Vibration Analysis :

- \* Two Natural Frequencies
- \* Two Mode Shapes (Natural Coordinate)

↳ Eigen value problem

\* Assume motion to be harmonic.

$$x_1 = \underline{X_1} \sin(\omega t) \rightsquigarrow \ddot{x}_1 = -\underline{X_1} \omega^2 \sin(\omega t)$$

$$x_2 = \underline{X_2} \sin(\omega t) \rightsquigarrow \ddot{x}_2 = -\underline{X_2} \omega^2 \sin(\omega t)$$

Substitute in the Equation of motion...

$$- \omega^2 \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} + \begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 + k_3 \end{bmatrix} \begin{bmatrix} X_1 \\ X_2 \end{bmatrix} \sin(\omega t) = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

عوضه در معادله حرکت  
و  $X_1$  و  $X_2$  را عامل مشترک

\* اذا كان  $X_1$  و  $X_2$  با يساوي  
صفر، معادلات القوس الثاني  
لازم يساوي صفر

doesn't equal  
Zero

$$\begin{bmatrix} k_1 + k_2 - \omega^2 m_1 & -k_2 \\ -k_2 & k_2 + k_3 - \omega^2 m_2 \end{bmatrix} \begin{bmatrix} X_1 \\ X_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Eigen Value problem, Determinate = 0

$$\begin{vmatrix} (k_1 + k_2) - \omega^2 m_1 & -k_2 \\ -k_2 & k_2 + k_3 - \omega^2 m_2 \end{vmatrix} = 0$$

$$\leadsto ((k_1 + k_2) - \omega^2 m_1) (k_2 + k_3 - \omega^2 m_2) - k_2^2 = 0$$

$$\leadsto (k_1 + k_2) (k_2 + k_3) - (k_1 + k_2) \omega^2 m_2$$

$$- m_1 \omega^2 (k_2 + k_3) + \omega^4 m_2 m_1 - k_2^2 = 0$$

↓

$$m_1 m_2 \omega^4 - \omega^2 (m_1 (k_2 + k_3) + m_2 (k_1 + k_2))$$

$$+ (k_1 k_2 + k_1 k_3 + k_2 k_3) = 0$$

Assume  $\lambda = \omega^2 \leadsto$  يعني نطلع عندي معادلة تربيعية

$$\rightarrow m_1 m_2 \lambda^2 - (m_1 (k_2 + k_3) + m_2 (k_1 + k_2)) \lambda$$

$$+ (k_1 k_2 + k_1 k_3 + k_2 k_3) = 0$$



$$\lambda_{1,2} = \frac{m_1(k_2 + k_3) + m_2(k_1 + k_2)}{2 m_1 m_2} \pm \sqrt{\frac{(m_1(k_2 + k_3) + m_2(k_1 + k_2))^2 - 4 m_1 m_2 (k_1 k_2 + k_1 k_3 + k_2 k_1)}{4 m_1^2 m_2^2}}$$

القانون العام ← كثير الحدود XD

$$\lambda_1 = \omega_{n1}^2 \longrightarrow \omega_{n1} = \sqrt{\lambda_1} \text{ rad/s}$$

$$\lambda_2 = \omega_{n2}^2 \longrightarrow \omega_{n2} = \sqrt{\lambda_2} \text{ rad/s}$$

Which are the two natural frequencies of the system

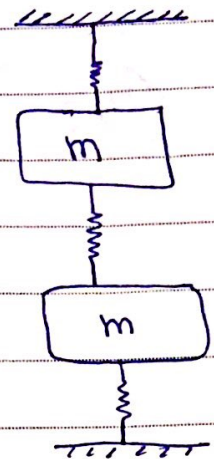
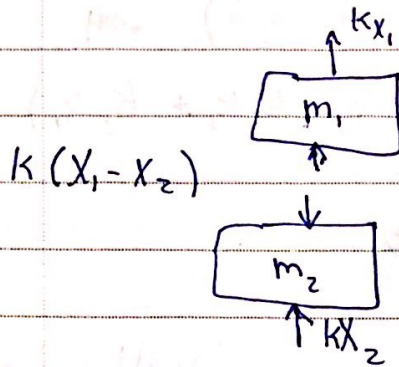
→ Now go back to eqn (\*) solve for

Vector which is the eigen vector  $\begin{Bmatrix} x_1^{(1)} \\ x_2^{(1)} \end{Bmatrix}$   
(mode shape)

Similarly, substitute for  $\omega_{n2}$  to solve for the eigen value which is the 2<sup>nd</sup> mode shape

Example

First we take the deformed shape..  
and define the forces acting on the 2 masses.



For the first mass :  $\sum F_{x_1} = m \ddot{x}_1 = -kx_1 - k(x_1 - x_2)$

$$\rightarrow m \ddot{x}_1 + kx_1 + k(x_1 - x_2) = 0$$

For the second mass :  $\sum F_{x_2} = m \ddot{x}_2 = -kx_2 + k(x_1 - x_2)$

$$+ \quad m \ddot{x}_2 + kx_2 + k(x_1 - x_2) = 0$$

$$m \ddot{x}_1 + 2kx_1 - kx_2 = 0$$

$$m \ddot{x}_2 + 2kx_2 - kx_1 = 0$$

$$\rightarrow \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} 2k & -k \\ -k & 2k \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$



it is Eigen Value Problem..

$$[ [K] - \omega^2 [m] ] [x] = 0$$



$$\begin{bmatrix} 2K - m\omega^2 & -K \\ -K & 2K - m\omega^2 \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = 0$$

$$\Delta(\omega^2) = \begin{vmatrix} 2K - m\omega^2 & -K \\ -K & 2K - m\omega^2 \end{vmatrix} = 0$$

$$(2K - m\omega^2)^2 - K^2 = 0$$

$$4K^2 - 4Km\omega^2 + m^2\omega^4 - K^2 = 0$$

$$m^2\omega^4 - 4Km\omega^2 + 3K^2$$

$$\text{assume } \omega^4 = \gamma^2$$

$$m^2\gamma^2 - 4Km\gamma + 3K^2 = 0$$

roots of  
المعادلة  
بطلب  
 $\omega_{n1}$  &  $\omega_{n2}$

$$\omega^2 = \frac{km \pm \sqrt{16k^2m^2 - 12m^2k^2}}{2m^2}$$

$$= \frac{4km \pm 2mK}{2m^2}$$

$$\omega_2^2 = \frac{6k}{2m} = \frac{3k}{2m} \leadsto \omega_2 = \sqrt{\frac{3}{2} \frac{k}{m}}$$

$$\omega_1^2 = \frac{2km}{2m^2} = \frac{k}{m} \leadsto \omega_1 = \sqrt{\frac{k}{m}}$$

Recall  $(2k - m\omega^2)x_1 - kx_2 = 0$

Substitute By  $\omega_1$ , and define  $r_1 = \frac{x_1}{x_2}$

$$r_1 = \frac{x_1}{x_2} = \frac{k}{2k - m\omega_1^2} = \frac{k}{2k - k \frac{k}{m}} = 1$$

هنا الرمز يعني  
يقابل مع  $\omega_1$

$$\begin{pmatrix} x_1^{(1)} \\ x_2^{(1)} \end{pmatrix} = \begin{pmatrix} x_1^{(1)} \\ \frac{x_1^{(1)}}{r_1} \end{pmatrix} = \begin{pmatrix} 1 \\ 1 \end{pmatrix}$$

هنا يعني يتحرك مع بعض  
 $m_1$   $\uparrow$   
 $m_2$   $\uparrow$



For the 2<sup>nd</sup> natural frequency.

$$(2K - m\omega_2^2) X_1^{(2)} - K X_2^{(2)} = 0$$

$$r_2 = \frac{X_2^{(2)}}{X_1^{(2)}} = \frac{2K - m\omega_2^2}{K}$$

$$= \frac{2K - m \frac{3K}{m}}{K} = \frac{-K}{K} = -1$$

$$\Rightarrow \begin{bmatrix} X_1^{(2)} \\ X_2^{(2)} \end{bmatrix} = \begin{bmatrix} X_1 \\ r_2 X_1 \end{bmatrix} = X_1 \begin{bmatrix} 1 \\ -1 \end{bmatrix}$$

$m_1 \downarrow$   
 $m_2 \uparrow$

يعني بحركوا عكس بعض

في المود الثاني  $m_1 = m_2$  يكون هاتين الحركتين عكس بعض  
 First mode  $m_1$   $m_2$   $\omega_1 = \sqrt{\frac{K}{m}}$

Second mode  $\omega_2 = \sqrt{\frac{3K}{2m}}$

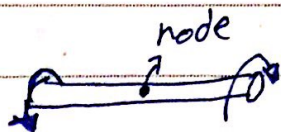
$m_1$   $m_2$

Node Point  $\Delta$  اي

(No motion)

(No Deflection)

Torsion example



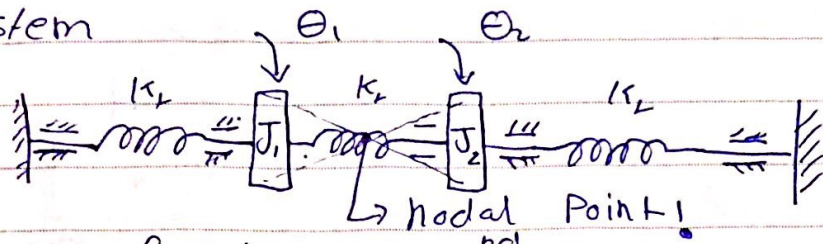
\* Rule:

No.

No. of nodes = Mode - 1  
 For each mode

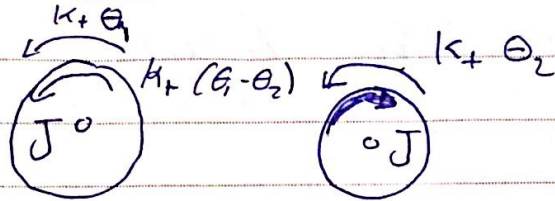


## Torsional System



انفصلنا ال node  
ببقرب على ال ال ال  
J اكبر

for Newtons 2<sup>nd</sup> law



(+)

$$\sum M_{J_1} = J_1 \ddot{\theta}_1 = -k_t \theta_1 - k_t (\theta_1 - \theta_2) = 0$$

$$J_1 \ddot{\theta}_1 + 2k_t \theta_1 - k_t \theta_2 = 0$$

$$\sum M_{J_2} = J_2 \ddot{\theta}_2 = -k_t \theta_2 + k_t (\theta_1 - \theta_2)$$

$$J_2 \ddot{\theta}_2 = -k_t \theta_2 + k_t (\theta_1 - \theta_2)$$

$$J_2 \ddot{\theta}_2 + 2k_t \theta_2 - k_t \theta_1 = 0$$

so

$$\begin{bmatrix} J_1 & 0 \\ 0 & J_2 \end{bmatrix} \begin{bmatrix} \ddot{\theta}_1 \\ \ddot{\theta}_2 \end{bmatrix} + \begin{bmatrix} 2k_t & -k_t \\ -k_t & 2k_t \end{bmatrix} \begin{bmatrix} \theta_1 \\ \theta_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

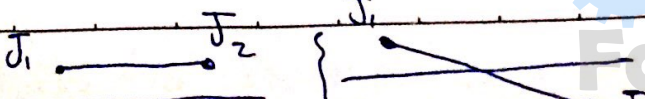
Same as Previous example!

$$W_1 = \sqrt{\frac{k_t}{J}}, \quad W_2 = \sqrt{\frac{3k_t}{J}}$$

$$\text{mode 1} = \begin{Bmatrix} 1 \\ 1 \end{Bmatrix}$$

$$\text{mode 2} = \begin{Bmatrix} 1 \\ -1 \end{Bmatrix}$$

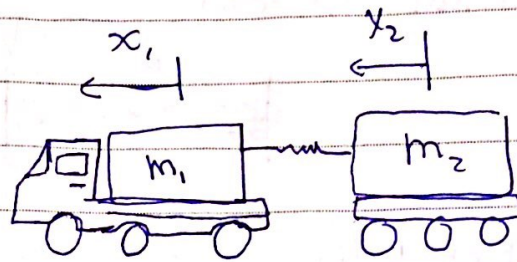
and Same nodes





Example ..

home work

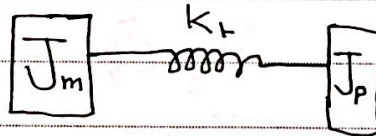


Find the natural frequencies and mode shapes.

\* From the solution, we can see that  $\omega_n = 0$ ,  $T_{\text{period}} = \infty$

which means that there is no vibration..

and this happen in torsional systems as in pumps and motor.



$$\begin{bmatrix} J_m & 0 \\ 0 & J_p \end{bmatrix} \begin{bmatrix} \ddot{\theta}_1 \\ \ddot{\theta}_2 \end{bmatrix} + \begin{bmatrix} k_t & -k_t \\ -k_t & k_t \end{bmatrix} \begin{bmatrix} \theta_1 \\ \theta_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

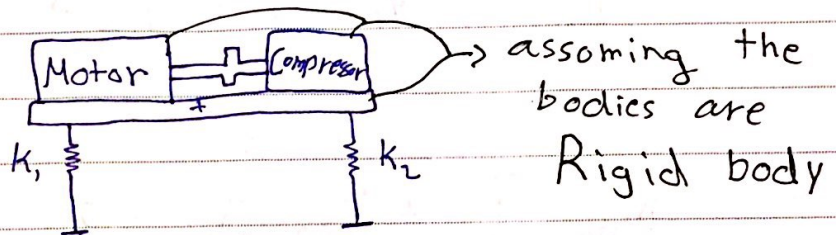
$$\Delta K = 0 !!$$

Due to rigid body mode...

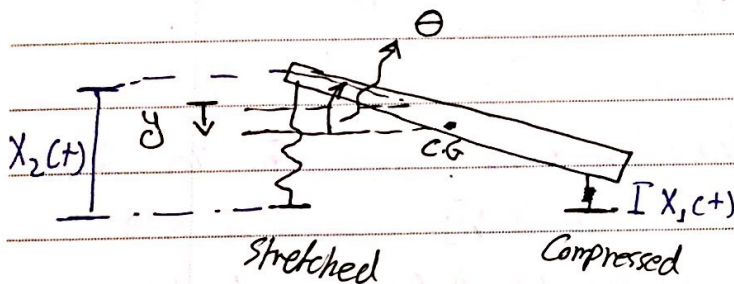
any 2DoF can be represented in a matrix

$$\begin{bmatrix} m_{11} & m_{12} \\ m_{21} & m_{22} \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Example:



First  $\rightarrow$  What are the degree of Freedom?  $\rightarrow \underline{2}$



\* بقدر أوقف الحركة بـ  
 (1) y (حركة الـ C.G. بشكل  
 عمودي)

Rotational Motion  $\leftarrow \theta$  (2)  
 حوليا الـ C.G.

x و بنفس الوقت بقدر أوقف هابي الحركة بـ  $x_1(t)$  &  $x_2(t)$   
 وهيك برينو 2DoF ..

بمعنى آخر

There must be some Coordinate system  $q_1(t)$  &  $q_2(t)$  that we can use and result a new system of equation (even if there is no Physical meaning) that will lead to uncoupled system which mean the two equation can be Solved independently. These

Coordinates are the Natural (Principle) Coordinates



\* Un Coupled  $\leadsto$ 

يعني مشا متكبلين يعني

ولا اسم نزل يعني، وبتهير ال matrix

$$\begin{bmatrix} m_{qq_1} & 0 \\ 0 & m_{qq_2} \end{bmatrix} \begin{bmatrix} \ddot{q}_1 \\ \ddot{q}_2 \end{bmatrix} + \begin{bmatrix} k_{qq_1} & 0 \\ 0 & k_{qq_2} \end{bmatrix} \begin{bmatrix} q_1 \\ q_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

$$\downarrow$$

$$m_{qq_1} \ddot{q}_1 + k_{qq_1} q_1 = 0 \quad q_1(t) = ?$$

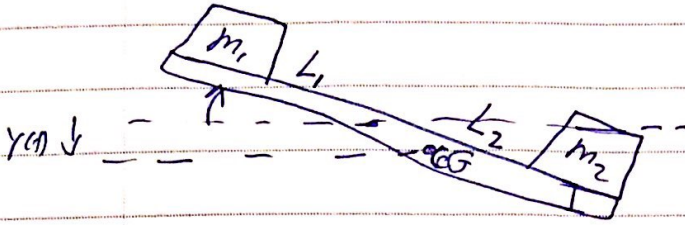
Independent

$$m_{qq_2} \ddot{q}_2 + k_{qq_2} q_2 = 0 \quad q_2(t) = ?$$

$\leadsto$  There must be a transformation that can transform  $[m]$  and  $[k]$  from  $[x]$  to  $[q]$

i.e From physical Coordinates to Natural Coordinates ...

Example: recall the motor and compressor  
and solve it with  $y(t)$  and  $\theta(t)$



$$x_1 = L_1 \theta - y$$

$$x_2 = y + L_2 \theta$$

Apply Newton's 2<sup>nd</sup> law

home work  $\rightarrow$  Find the Equation of motion



Recall the 2 DoF Free Vibration Matrix:

$$\begin{bmatrix} m_{11} & m_{12} \\ m_{21} & m_{22} \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

→ Eigen Value problem →  $\omega_1, \omega_2$

$$\begin{bmatrix} x_1^{(1)} & x_1^{(2)} \\ x_2^{(1)} & x_2^{(2)} \end{bmatrix} \rightarrow \text{Modal Matrix } [V]$$

$$\Rightarrow [m_{\text{new}}] = [V]^T [m] [V]$$

$$= \begin{bmatrix} m_{1\text{new}} & 0 \\ 0 & m_{2\text{new}} \end{bmatrix}$$

Let  $[V]^T$  و  $V$  ب  $V$  Diagonal Matrix  
بطيني و لا يلاو لاه لاه لاه  
Uncoupled equations.

→ Similarly for  $K$

$$[K_{\text{new}}] = [V]^T [K] [V]$$

Swap between Rows and Columns

$$= \begin{bmatrix} k_{1\text{new}} & 0 \\ 0 & k_{2\text{new}} \end{bmatrix}$$

that will give Modal equations that are uncoupled where each of them can be solved independently from the other with new coordinate  $\begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}$  and are called the natural coordinates.

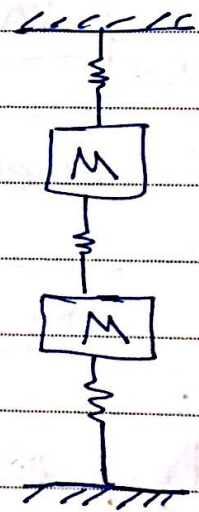
$$\begin{bmatrix} m_{1, \text{new}} & 0 \\ 0 & m_{2, \text{new}} \end{bmatrix} \begin{bmatrix} \ddot{q}_1 \\ \ddot{q}_2 \end{bmatrix} + \begin{bmatrix} k_{1, \text{new}} & 0 \\ 0 & k_{2, \text{new}} \end{bmatrix} \begin{bmatrix} q_1 \\ q_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

$$\Rightarrow \text{Uncoupled} \Rightarrow m_{1, \text{new}} \ddot{q}_1 + k_{1, \text{new}} q_1 = 0$$

$$m_{2, \text{new}} \ddot{q}_2 + k_{2, \text{new}} q_2 = 0$$

\* this can be applied to any undamped multi degree of freedom systems (check)

H.W on Wednesday (Very Important to be studied and solved)



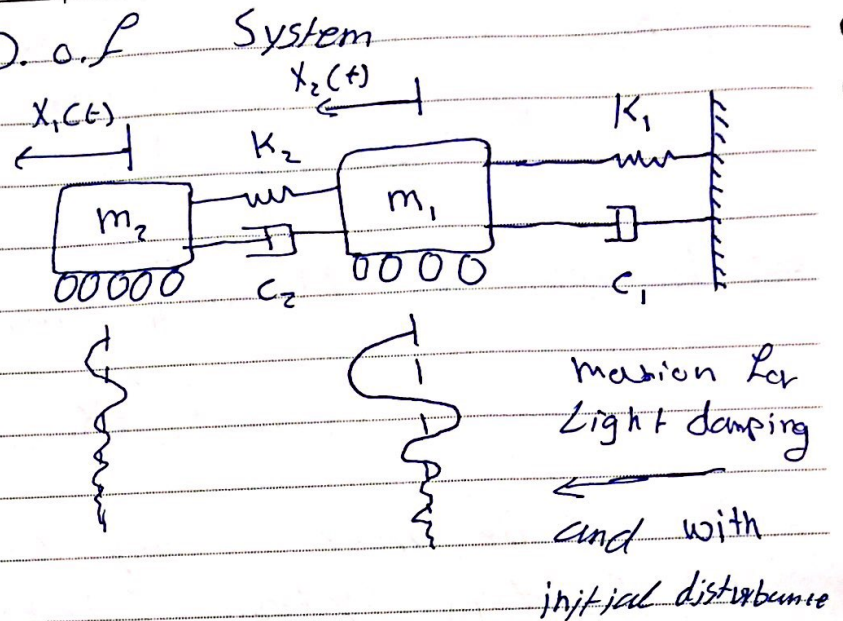
- 1 Formulate Equation of motion
- 2 Find natural frequencies
- 3 Find modal matrix
- 4 Uncouple equations
- 5 get final uncoupled equations

$$\begin{aligned} K &= 100 \text{ N/m} \\ m &= 1 \text{ kg} \end{aligned}$$

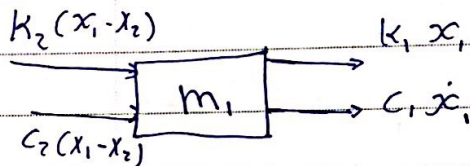


# \* Damped 2 D.o.f System

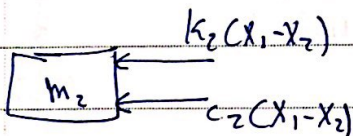
- Formulate the model  
Equation of motion  
→ Newton's Laws



F.B.D



and



## \* Assumptions:

- Linear Springs
- Linear dampers
- Small motion

So the Equation of motion

→ Apply Newton's 2nd law

$$\sum F_{x_1} = m_1 \ddot{x}_1 = -k_1 x_1 - c_1 \dot{x}_1 - k_2(x_1 - x_2) - c_2(\dot{x}_1 - \dot{x}_2)$$

$$\leadsto m_1 \ddot{x}_1 + (c_1 + c_2) \dot{x}_1 + c_2 \dot{x}_2 + (k_1 + k_2) x_1 - k_2 x_2 = 0$$

$$\sum F_{x_2} = m_2 \ddot{x}_2 = c_2(\dot{x}_1 - \dot{x}_2) + k_2(x_1 - x_2)$$

$$\leadsto m_2 \ddot{x}_2 + c_2 \dot{x}_2 - c_2 \dot{x}_1 + k_2 x_2 - k_2 x_1 = 0$$

the matrix for these 2 equation is

$$\underbrace{\begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix}}_{\text{mass matrix}} \underbrace{\begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix}} + \underbrace{\begin{bmatrix} c_1+c_2 & -c_2 \\ -c_2 & c_2 \end{bmatrix}}_{\text{damping matrix}} \underbrace{\begin{bmatrix} \dot{x}_1 \\ \dot{x}_2 \end{bmatrix}} + \underbrace{\begin{bmatrix} k_1+k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix}}_{\text{stiffness matrix}} \underbrace{\begin{bmatrix} x_1 \\ x_2 \end{bmatrix}} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Now what is the Eigen Value problem for this system?

Recall  $\rightarrow$  Eigen Value  $\rightarrow$  Natural frequencies  
Eigen Vector  $\rightarrow$  Mode shapes..

$\lambda = j\omega$   $\rightarrow$   $\lambda$  is complex  $\leftarrow$  Damping  $\lambda$  is  $\omega$   $\leftarrow$  Imaginary  
Real number  $\rightarrow$   $\omega_n$   $\leftarrow$  First order  $\rightarrow$  2nd order

$\Rightarrow$  to find eigen values / eigen vectors  $\Rightarrow$  1<sup>st</sup> order system

$$[m][\ddot{x}] + [c][\dot{x}] + [k][x] = [0]; \quad x = \begin{bmatrix} x_1 \\ x_2 \end{bmatrix}$$

$\begin{Bmatrix} y_{1, \text{new}} \\ y_{2, \text{new}} \end{Bmatrix} \rightarrow$  new Coordination System such that  $y_1 = x_1$   
 $y_2 = \dot{x}_1 \leftarrow y_3 = \dot{x}_2$   
 $y_4 = \dot{x}_2 \leftarrow y_4 = \dot{x}_2$



$$* [m^{-1}] * [m] \rightsquigarrow \text{Identity } [I]$$

$$[m]^{-1} \rightarrow \text{أعرب } *$$

$$\rightsquigarrow [ \ddot{x} ] + [m]^{-1} [c] \{ \dot{x}_i \} + [m]^{-1} [k] \{ x_i \} = \{ 0 \}$$



$$* \dot{y}_1 = y_2$$

$$\dot{y}_3 = y_4$$

~~$$\dot{y}_2 = y_3$$~~

$$\begin{Bmatrix} \dot{y}_1 \\ \dot{y}_2 \\ \dot{y}_3 \\ \dot{y}_4 \end{Bmatrix} = \begin{bmatrix} I & 0 \\ -[m]^{-1}[k] & -[m]^{-1}[c] \end{bmatrix} \begin{Bmatrix} y_1 \\ y_2 \\ y_3 \\ y_4 \end{Bmatrix}$$

$$\downarrow$$

$$[A]$$

Check the Numerical Example \*

# Forced Vibration (Harmonic excitation)

$$\begin{bmatrix} m_{11} & m_{12} \\ m_{21} & m_{22} \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} c_{11} & c_{12} \\ c_{21} & c_{22} \end{bmatrix} \begin{bmatrix} \dot{x}_1 \\ \dot{x}_2 \end{bmatrix} + \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} F_0 e^{j\omega t} \\ 0 \end{bmatrix}$$

$\omega$  is the Forcing Frequency

and Output follows the input

$$x_1(t) = x_1 e^{j\omega t}$$

$$x_2(t) = x_2 e^{j\omega t} \quad \text{and} \quad \dot{x}_1 = x_1 j\omega e^{j\omega t} \quad \text{and} \quad \ddot{x}_1 = -x_1 \omega^2 e^{j\omega t}$$

$$x_2 = x_2 j\omega e^{j\omega t} \quad \text{and} \quad \dot{x}_2 = -x_2 \omega^2 e^{j\omega t}$$

↓  
 \*  $x_1$  و  $x_2$  على شكل  $e^{j\omega t}$   
 \*  $\dot{x}_1$  و  $\dot{x}_2$  على شكل  $j\omega e^{j\omega t}$   
 \*  $\ddot{x}_1$  و  $\ddot{x}_2$  على شكل  $-\omega^2 e^{j\omega t}$

$$\begin{bmatrix} k_{11} - m_{11}\omega^2 + j c_{11}\omega & k_{12} - m_{12}\omega^2 + j c_{12}\omega \\ k_{21} - m_{21}\omega^2 + j c_{21}\omega & k_{22} - m_{22}\omega^2 + j c_{22}\omega \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} F_0 \\ 0 \end{bmatrix}$$

$$Z_{rs} = k_{rs} - m_{rs}\omega^2 + j c_{rs}\omega \quad \text{can be written as}$$

$$\begin{bmatrix} Z_{11} & Z_{12} \\ Z_{21} & Z_{22} \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} F_0 \\ 0 \end{bmatrix} \Rightarrow \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = [Z]^{-1} \begin{bmatrix} F_0 \\ 0 \end{bmatrix}$$

وهنا يوجد  $x_1$  و  $x_2$

→ Mechanical Impedance



## Harmonic Excitation (Steady State)

Another example:

and we assumed that the output follows

the input  $\Rightarrow x_1(t) = X_1 \cos(\omega t)$

$x_2(t) = X_2 \cos(\omega t)$

and reached for the impedance matrix

$$\begin{bmatrix} Z_{11} & Z_{12} \\ Z_{21} & Z_{22} \end{bmatrix} \begin{bmatrix} X_1 \cos(\omega t) \\ X_2 \cos(\omega t) \end{bmatrix} = \begin{bmatrix} F_0 \\ 0 \end{bmatrix}$$

↓

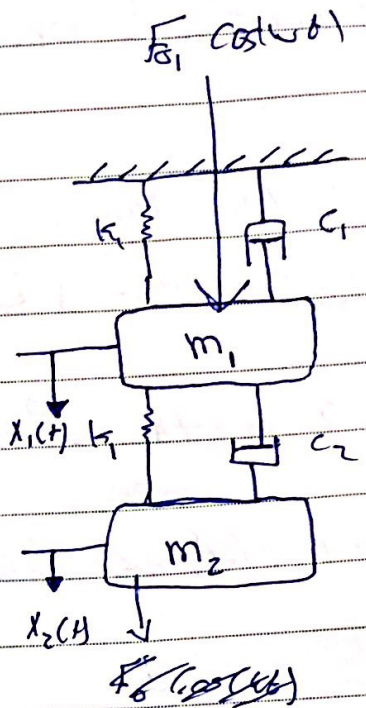
$$\begin{bmatrix} X_1 \cos(\omega t) \\ X_2 \cos(\omega t) \end{bmatrix} = [Z]^{-1} [F]$$

$$\Rightarrow Z_{rs} = k_{rs} - m_{rs} \omega^2 + i C_{rs} \omega$$

$$\Rightarrow [Z]^{-1} = \frac{1}{Z_{11}Z_{22} - Z_{12}Z_{21}} \begin{bmatrix} Z_{22} & -Z_{12} \\ -Z_{12} & Z_{11} \end{bmatrix}$$

المحدد

Determinant



$$X_1(i\omega) = \frac{Z_{22} F_{01}}{Z_{11}^2 - Z_{12} Z_{22}}$$

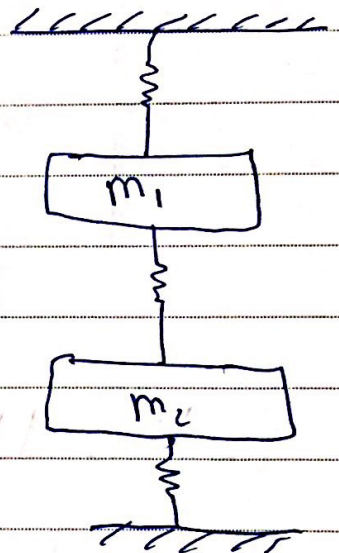
and

$$X_2(i\omega) = \frac{-Z_{12} F_{01}}{Z_{11}^2 - Z_{12} Z_{22}}$$

Example 8 | مثال 8 | بالكتاب (homework نفس ال)

Solution:

$$\left. \begin{array}{l} m_{12} = m_{21} = 0 \\ m_{11} = m_{22} = m \end{array} \right\} \begin{array}{l} C_{ij} = 0 \\ k_{11} = k_{22} = 2k \\ k_{12} = k_{21} = -k \end{array}$$



the impedance is

$$Z_{11} = 2k - m\omega^2$$

$$Z_{12} = Z_{21} = -k$$

$$Z_{22} = 2k - m\omega^2$$

$$\text{So } \Rightarrow X_1(i\omega) = \frac{(2k - m\omega^2) F_{01}}{k^2 - (2k - m\omega^2)^2}$$

$$X_2(i\omega) = \frac{k F_{01}}{k^2 - (2k - m\omega^2)^2}$$

$$\left. \begin{array}{l} \omega_1 = \sqrt{\frac{k}{m}} \\ \omega_2 = \sqrt{\frac{3k}{m}} \end{array} \right\}$$

Mech  
Family



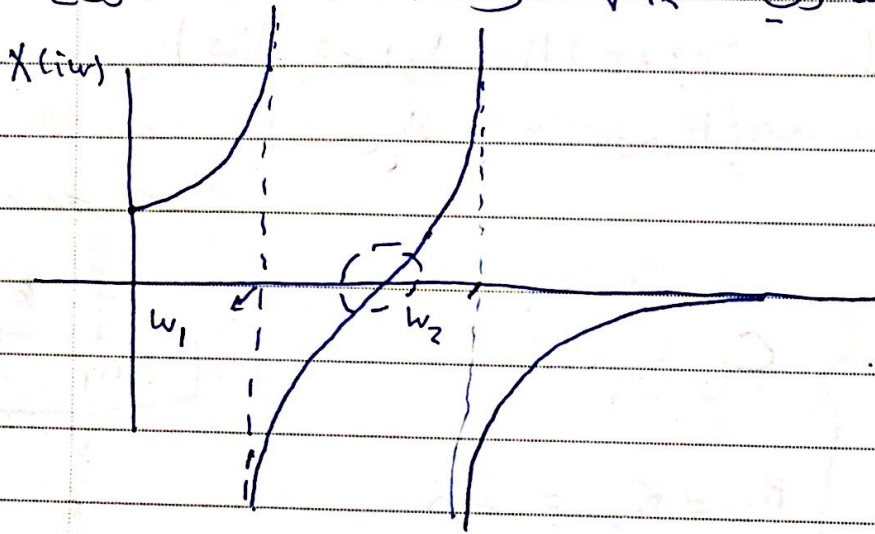
بدن ترسها ... کلیه انشوف المقام

$$k^2 - (2k - m\omega^2) = 0 \leadsto \omega = \sqrt{\frac{k}{m}} \rightarrow \infty$$

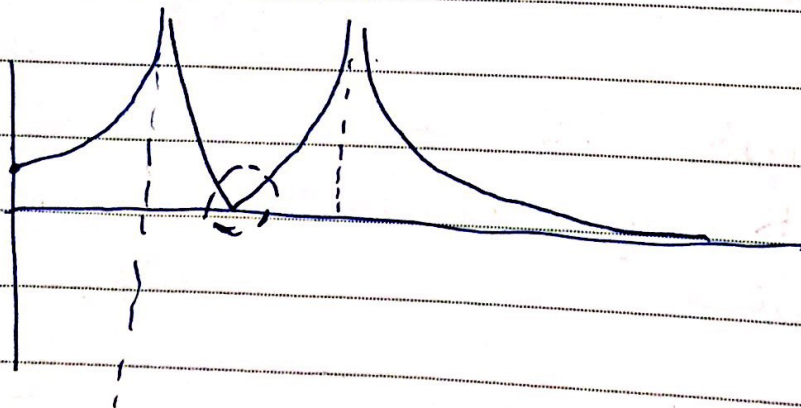
and

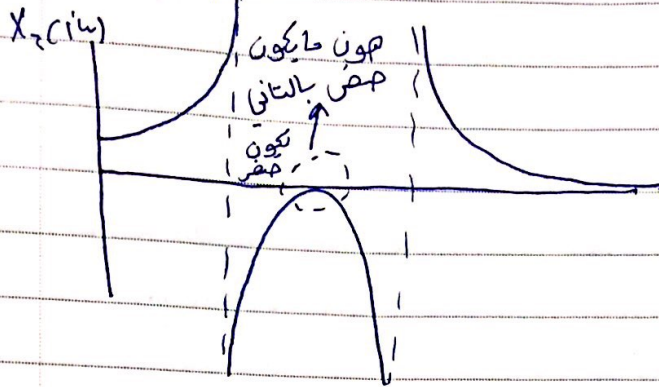
$$\omega = \sqrt{\frac{3k}{m}}$$

فما لا ٢ يساوي  $\sqrt{\frac{k}{m}}$  و  $\sqrt{\frac{3k}{m}}$  بزوج لا  $\infty$



أو ازا كانت  $|x_1|$  (مطلق)





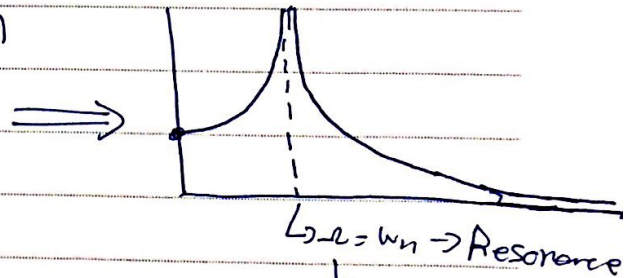
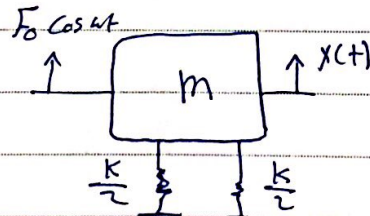
\* أهم ملاحظة يجهول الرستين انه في منطقة بينه قيه اد  
ليساوي مرض سوء وضهون حارينا اشى يستعمله اسمو

## Dynamic Vibration Absorber idea

\* من خلال اني أعلق  $mass$  معيه عند ال  $r$  اللي  
بشغل عليها وبخفيها ايا ه ...

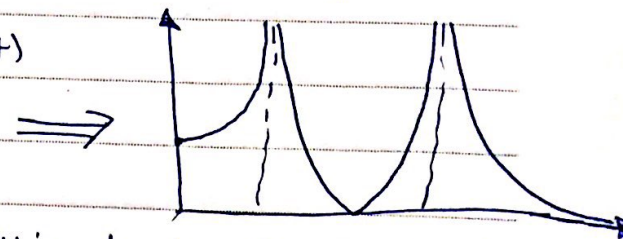
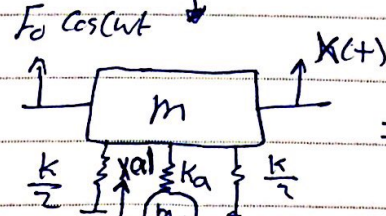
Chapter 9: Vibration Control

### \* Example



بدي أعلق ال mass و هيا

2 DoF  $\mu$  و  $\sigma$



ہکون بس (ma) بتھری

هبل مار عسي 2  $m$  وبار عدا الي يدي اياه + اصفه الي أنا شفا عسي ...

\* if Absorber Frequency is tuned to be equal to the external excitation frequency i.e. Amplitude will remain Primary

check the Rector Paper (Dual dynamic absorber 1999)



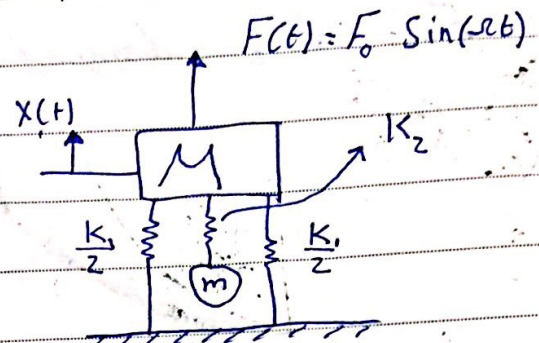
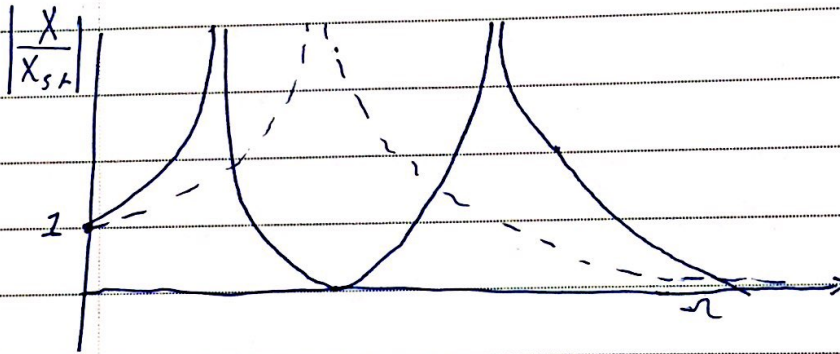
## Dynamic absorber

→ For undamped system

$$m_1 \ddot{x}_1 + k x_1 = F_0 \sin(\omega t)$$

where  $\omega$  is the forcing frequency

$$\omega \rightarrow \omega_1 \rightarrow \omega_1 = \sqrt{\frac{k}{m}}$$



\* if we add this mass, the system become 2 DOF

$$m_1 \ddot{x}_1 + k_1 x_1 + k_2 (x_1 - x_2) = F_0 \sin(\omega t)$$

$$m_2 \ddot{x}_2 + k_2 (x_2 - x_1) = 0$$

$$m_1 (-\omega^2 X_1 \sin \omega t) + k_1 X_1 \sin \omega t + k_2 (X_1 - X_2) \sin \omega t = F_0 \sin \omega t$$

the Steady state response:

$$X_1(t) = X_1 \sin \omega t$$

$$X_2(t) = X_2 \sin \omega t$$

So:

$$X_1 = \frac{(k_2 - m_2 \omega^2) F_0}{\Delta}$$

&

$$X_2 = \frac{k F_0}{\Delta}$$

$$\ddot{x}_1 = -\omega^2 X_1 \sin \omega t$$

$$\ddot{x}_2 = -\omega^2 X_2 \sin \omega t$$

نفس الشيء

# Dual Dynamic absorber

No. \_\_\_\_\_

\* after adding  $(m_a, k_a)$  System become 2 DoF System

→ Vibration is a form of Energy...

So it will go to the system even if it is close to resonance (by the easiest way which mean to the added mass)

\* if absorber frequency is tuned to be equal to the external excitation frequency, so the amplitude will remain at primary.

