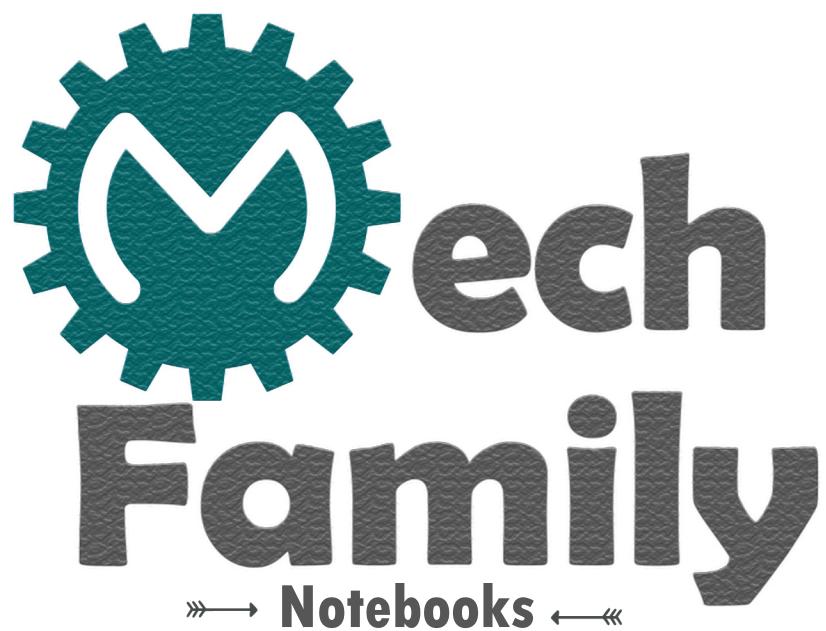


Vibrations

Dr. Basem Bdoor

1st Semester 2017

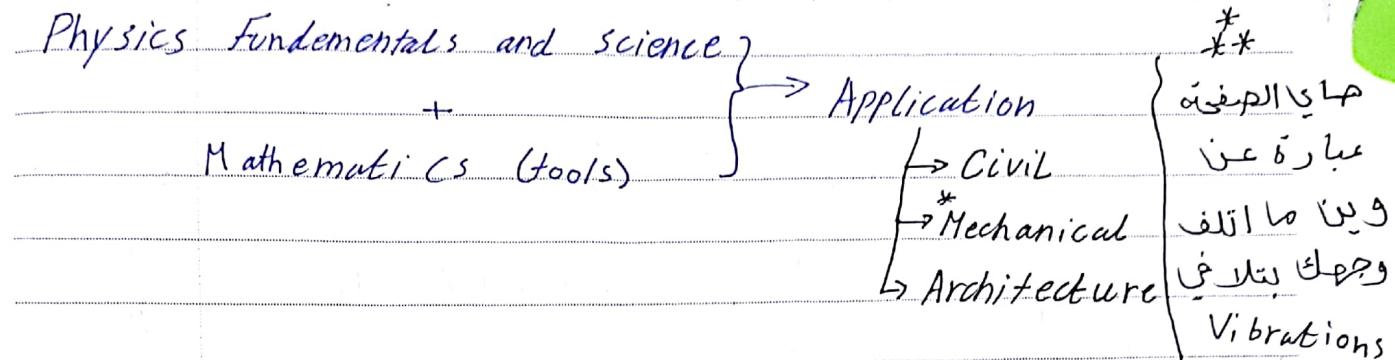


Vibrations

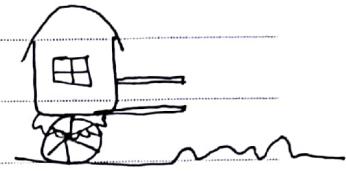
Dr. Basem Al. bdour

1st Semester (2017-2018)

No. Introduction



→ first Application was at horse Cart
 as the cart was held by leaf Spring
 as it became flexible element and
 absorb the effect of road irregularities



* Tire (Rubber with Air inside) → flexible elements

→ Thermal / Fluids

* Mechanical → Applied Mechanics → strength

Statics

* FYI: Electric Motor efficiency = $\frac{\text{Output (Mech)}}{\text{Input (elect)}}$
 $= \frac{T \times w}{I \times V}$

Dynamics

Design

Machines

→ Since efficiency is not 100%.

that means there is energy losses

Like the Vibration energy

all of them
 are related
 to

→ Tacoma bridge Phenomena! Mech. Vibrations!

↳ big failure due to vibration

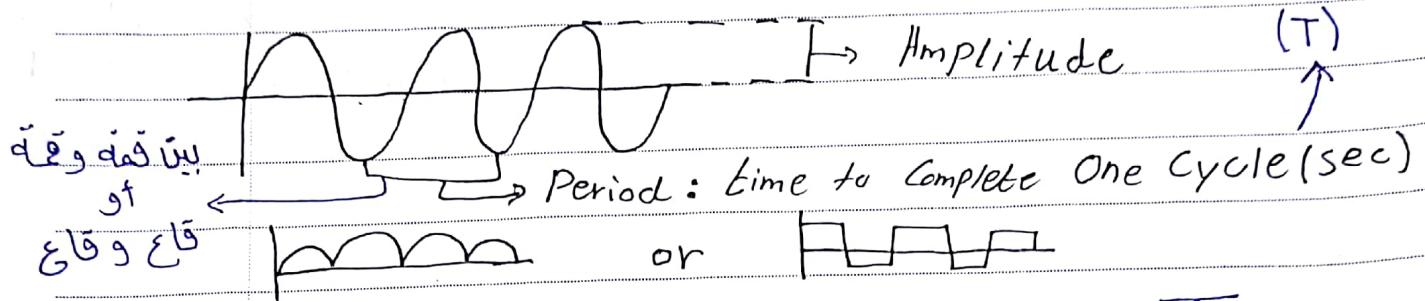
created by wind at certain speed.

* Vibration needs to be considered else \Rightarrow no comfort
 \Rightarrow Health Problem

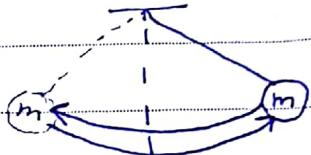
Hearing is Vibration, microphone is Vibration!, Sound isolation!,

Vibration: repeated motion (positional) at period of time!

⇒ it can be represented by sinusoidal wave!



example: The Pendulum Motion



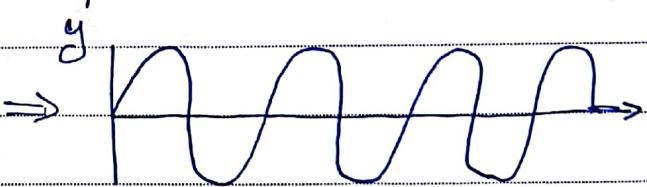
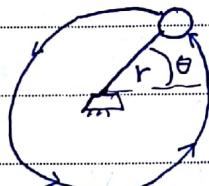
in electric field → Period is $\frac{1}{50000}$ sec
(hertz)

* Frequency (Hz) = $\frac{1}{T}$
or
cycles per sec (cps)

$$y = r \sin \theta$$

المسافة العمودية

* example:



$$\phi = \omega t = 2\pi f t$$

$$\omega = 2\pi f$$

* The material is available on Website (Slide share.net)

↳ Search in mechanical vibration Slide Share

* Starting Potential energy Types: ① gravity (height)

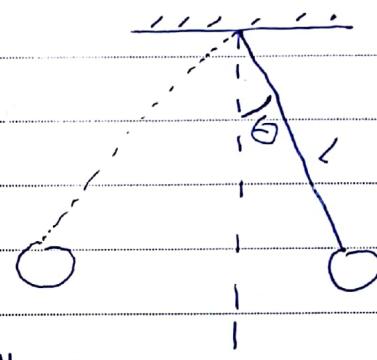
② elasticity (Springs)

elasticity (no Stiffness) \longleftrightarrow

* Simplest Type of Free vibration \rightarrow The pendulum.

KE is zero at Maximum height

$$mgL()$$



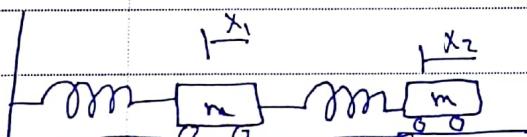
* The 3 elements of vib system: ① mass

② spring

③ damper

* degree of freedom \rightarrow درجه الحرية

النقطة المهمة هي



* 2 degree of freedom

(2 Single and independent
degree of freedom
for each part)

Vibration Classification

* Free or Forced { Damped & undamped

Linear and Non-Linear



Differential Equation

↳ معادلة дифференциальная

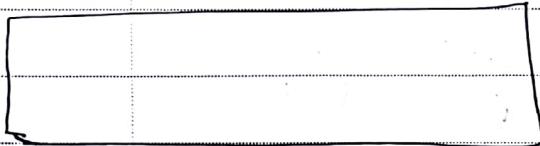
{ Deterministic or Random
(Predictable / Not Predictable)

* There is Assumption must be taken, then formulate and Analysing and Differentiation in order to find the Vibration Results (^{System}Characteristic)

* Spring Element:

*For a Cantilever beam:-

the deflection at Free end is



and take the Force and
treat like a spring

so you can find the
spring constant K

and

$$\text{Springs in series} \rightarrow \frac{1}{K_{\text{eq}}} = \frac{1}{K_1} + \frac{1}{K_2} + \frac{1}{K_3} \dots$$

Springs in parallel $\rightarrow K_{eq} = k_1 + k_2 + k_3$

$$F = k * \delta \quad \text{حالات مراجعة} \quad \text{(Check the Slides)}$$

بندر و نظم لیش

- **Vibration:** Motion that repeats itself at certain pattern

- **Free vibration:** Initial disturbance (excitation)

Vibrates freely, continuous exchange between (KE & PE)

* for the same material, Area, Cross-section
the speed of vibration doesn't change.

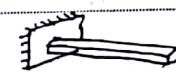
* Change in length \rightarrow Change in Stiffness (K)

- **Forced vibration:** under continuous input from the outside of the system

- **Degree of Freedom (DoF):**

- Continuous system (∞ DoFs) ex: Beam

all coordinates have 1 DoF *

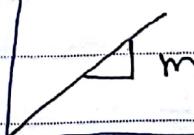


..... (X, Y, Z...) \rightarrow ∞ DoFs

- Discrete system (finite DoF's) ex: Single DoF

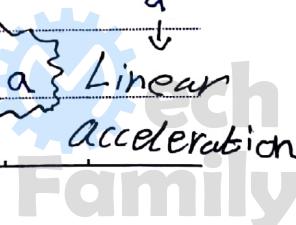
- **Damping:** - Damped ($\frac{F_d}{F_0}$ or Friction)
- Undamped \rightarrow No Energy dissipation

* The system that we deal with is: F



Mass element \rightarrow Store KE

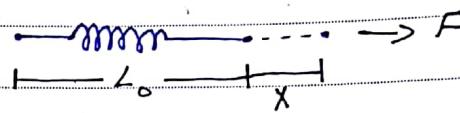
* Inertia is Body Resistance to acceleration. $F = m a$ Linear acceleration



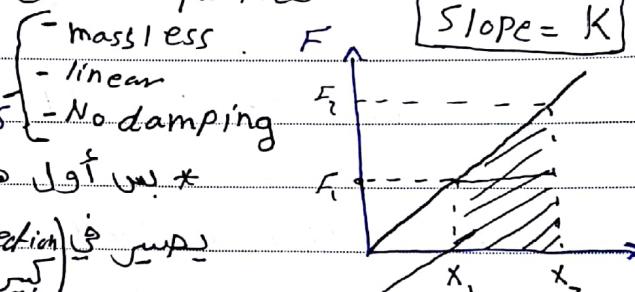
- For Angular acceleration $\alpha \rightarrow M = J \ddot{\theta} = J d$ (rad/s²)

* Ideal Point mass, no dimensions, }
 - No Spring effect, No damping } $m = m_1 + m_2 + \dots + m_n$

2 Spring element : (Store PE)

↳ transmits all forces 

- gives flexibility for Suspension System and resist the system inputs like earth quakes.

- Slope = K = Stiffness 

↳ Linear Hardening spring (deflection is \propto force)

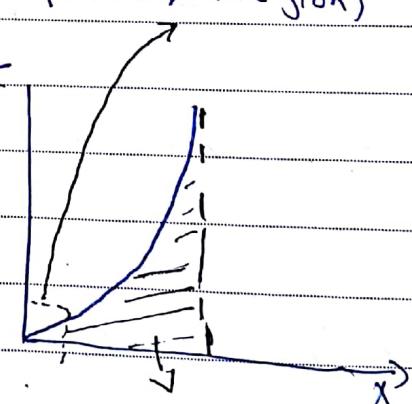
- So in this Course we will deal with the linear region only for development and Predict system behavior.

- The area is Energy Stored in Spring

$$E = \frac{1}{2} K X^2$$

$$F = K X$$

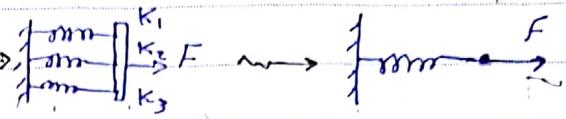
integrate
From Force to energy



"hardening"

*Spring Connections :

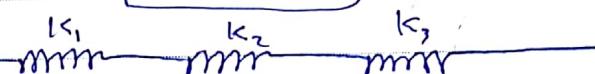
- Spring in Parallel



$$F = F_1 + F_2 + \dots + F_n \rightarrow K_{eq} * x = K_1 x_1 + K_2 x_2 + \dots + K_n x_n \quad \{ x = x_1 = x_2 = \dots \}$$

$$K_{eq} = \sum K_n$$

- Spring in Series:



$$x_{tot} = x_1 + x_2 + x_3 + \dots + x_n \rightarrow \frac{F}{K_{eq}} = \frac{F_1}{K_1} + \frac{F_2}{K_2} + \dots + \frac{F_n}{K_n}$$

* يعني عكس المقاويم التي
بالكثير باردة

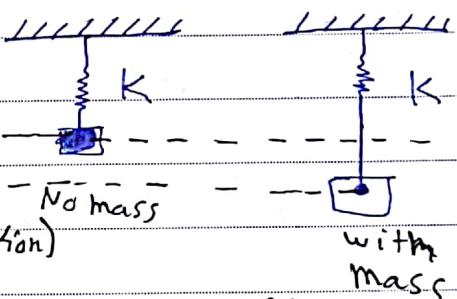
$$K_{eq} = \sum \frac{1}{K_n}$$

mmmm

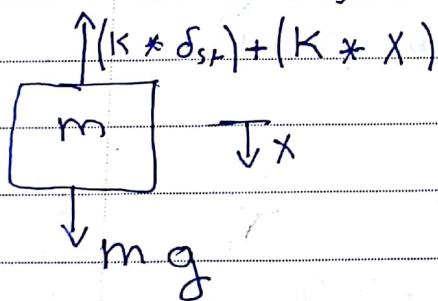
* هي الحاله اسفل

"Static equilibrium Position"

$$mg = K \delta_{st} \quad \text{لي عنده} \quad (\text{Static deflection})$$



* F.B.D & apply Newton's 2nd law:



$$\sum F_x = m \ddot{x} = mg - K \delta_{st} - Kx$$

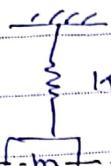
$$m \ddot{x} + Kx = 0$$

(2nd ODE, homogeneous, constant coefficient)

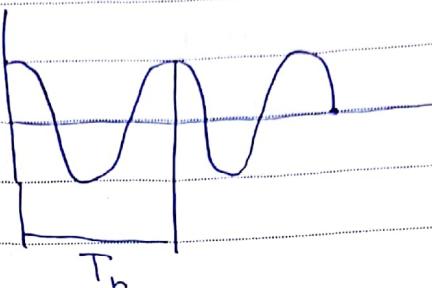
Initial Condition

are needed (x_0, \dot{x}_0)

Initial Condition



'Static equilibrium Position'



$$K x_0 = mg$$

$$m\ddot{x} + Kx = 0 \rightarrow x(0) = x_0, \dot{x}(0) = v_0$$

→ Assume $x(t) = A e^{i\lambda t}$ and from the solution of
1st ODE, homogeneous, Linear

$$\dot{x}(t) = A i\lambda e^{i\lambda t}$$

$$\ddot{x}(t) = A \lambda^2 e^{i\lambda t}$$

Substitute in equation

$$m\lambda^2 A e^{i\lambda t} + K A e^{i\lambda t} = 0$$

↓

$$(m\lambda^2 + K) A e^{i\lambda t} = 0 \rightarrow m\lambda^2 + K = 0$$

$$\lambda = \pm \sqrt{-\frac{K}{m}} = \pm i \sqrt{\frac{K}{m}}$$

$$\Rightarrow x(t) = B_1 \cos\left(\sqrt{\frac{K}{m}} t\right) + B_2 \sin\left(\sqrt{\frac{K}{m}} t\right)$$

+ The Natural frequency (ω_n) = $\frac{2\pi}{T_n}$

$$f = \frac{1}{T_n}$$

(Hz)

natural period

$$= 2\pi f$$

cos & sin λt , $\lambda = \pm i \sqrt{\frac{K}{m}}$

$$e^{i\lambda t} = \cos \lambda t + i \sin \lambda t$$

Euler's Identity $\lambda = i\omega$

$$x(t) = B_1 \cos(\omega_n t) + B_2 \sin(\omega_n t)$$

$$= B_1 \cos\left(\frac{2\pi}{T} t\right) + B_2 \sin\left(\frac{2\pi}{T} t\right)$$

$$= B_1 \cos 2\pi f t + B_2 \sin(2\pi f t)$$

* B_1 and B_2 are found by The initial Condition

$$\rightarrow x(0) = B_1 + 0 = x_0$$

$$\dot{x}(0) = v_0$$

$$x(0) = x_0$$

$$\rightarrow B_1 = x_0$$

$$\rightarrow \dot{x}(0) = 0 + B_2 \omega_n = v_0$$

$$B_2 = \frac{v_0}{\omega_n}$$

الحل general

Complementary Solution

\rightarrow So The general Solution is

$$x_{gs} = x_{cs} + x_{PI}$$

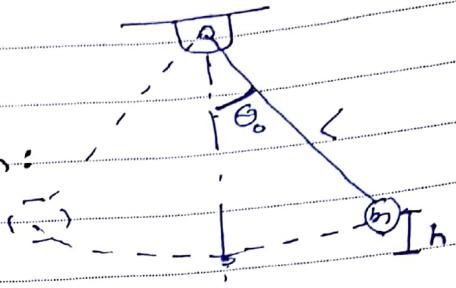
Particular Integral

$$x_{gs} = x_{cs} = x_0 \cos \omega_n t + \frac{v_0}{\omega_n} \sin \omega_n t$$

الحل general يكون
الطرف الثاني (البعض)
ما يساوي

Simple Pendulum Example:

Let's take a Free body diagram:



* we need to use Equation with
The Variable θ

* by parallel axis Theorem

J_0 (mass moment of inertia)

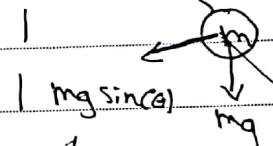
F.B.D

here

$$J_0 = m L^2$$

(For simple
Pendulum)

$$I_{G(H)}$$



\Rightarrow The Equation of Motion:

Tangent $\approx L\ddot{\theta}$

$$m L^2 \ddot{\theta} + mg L \sin(\theta) = 0$$

2nd ODE, Non-linear (sin), homogeneous

Normal

\Rightarrow By Taylor Expansion and Assuming $\sin \theta \approx \theta$

(Small Angle)

$$\rightarrow m L^2 \ddot{\theta} + mg L \theta = 0$$

{

$$L \ddot{\theta} + g \theta = 0 ; g = 9.81 \text{ m/s}^2$$

Recall the Equation of the Previous Example:

$$m\ddot{x} + kx = 0 \Rightarrow \ddot{x} + \frac{k}{m}x = 0 \Rightarrow \ddot{x} + \omega_n^2 x = 0$$

لدينا نحن نفس الاشي بعادلة او

$$\ddot{\theta} + \frac{g}{L} \theta = 0$$

$$\Rightarrow \omega_n^2 \Rightarrow \omega_n = \sqrt{\frac{g}{L}}$$

* ادا الاشي الوحيد الذي يتحكم بالعادلة هو الطول (L)
بالنها نفس العادلة قنبل نفس طريقة سؤال ا

$$\Rightarrow \theta(t) = \theta_0 \cos(\omega_n t) + \frac{\dot{\theta}_0}{\omega_n} \sin(\omega_n t)$$

$$\omega_n = \sqrt{\frac{g}{L}}$$

* Energy Conservation of Energy (طريقة تاثير المعدل)

$$P.E = \frac{1}{2} kx^2, K.E = \frac{1}{2} m \dot{x}^2$$

$$\text{So} \rightarrow \frac{1}{2} m \dot{x}^2 + \frac{1}{2} kx^2 = \text{Constant} \downarrow$$

↓ taking The derivatives



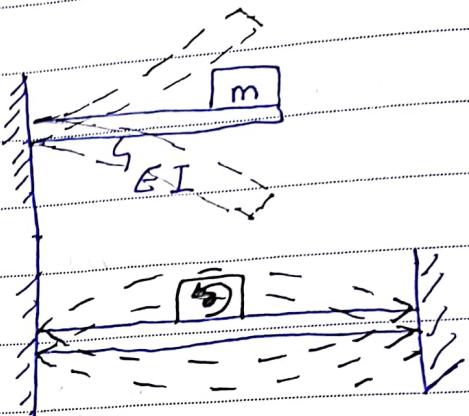
$$m\ddot{x}\dot{x} + kx\dot{x} = c \Rightarrow m\ddot{x} + kx = 0$$

Some Procedure for Simple pendulum:

$$KE = \frac{1}{2} I \dot{\theta}^2, PE = mg(\text{height}) = mg(L - L \cos \theta)$$

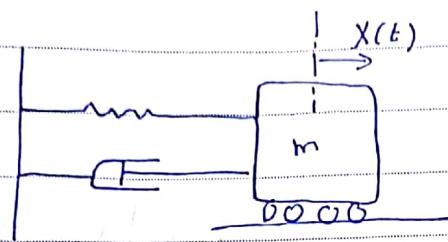
$$\Rightarrow \frac{d}{dt} \left(\frac{1}{2} I \dot{\theta}^2 \right) + mg(L - L \cos \theta) = \text{Constant}$$

Example \rightarrow Cantilever Beam



* Quiz \rightarrow next week, read the first chapter (1)

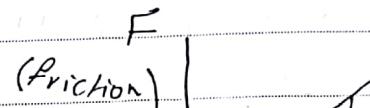
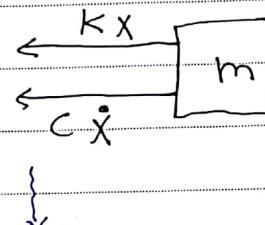
Free Vibration of SDOF "Viscous Damping"



Damper \rightarrow يكون في الموضع
بتأثير بقوة اعتماد على
 \dot{x} (زوج الموضع)

drawing The

F. B. D



$$\sum F_x = m \ddot{x}$$

$$m \ddot{x} = -c \dot{x} - k x \quad \boxed{x(0) = x_0, \dot{x}(0) = \dot{x}_0 \rightarrow I.C's}$$

\hookrightarrow 2nd O.D.E with Constant Coefficients, homogeneous

assume $x(t) = A e^{\lambda t}$

$$\begin{aligned} \dot{x}(t) &= A \lambda e^{\lambda t} \\ \ddot{x}(t) &= A \lambda^2 e^{\lambda t} \end{aligned} \quad \left. \begin{aligned} &\text{Substitute in the previous} \\ &\text{Equation} \end{aligned} \right\}$$

$$\rightarrow \text{The Characteristic equation} \rightarrow m \lambda^2 + c \lambda + k = 0$$

The roots are

$$\lambda_{1,2} = \frac{-c \pm \sqrt{c^2 - 4mk}}{2m}$$

$$\frac{-c}{2m} \pm \sqrt{\frac{c^2 - 4mk}{4m^2}}$$

So the roots are:

$$\frac{-c}{2m} + \sqrt{\frac{c^2}{4m^2} - \frac{k}{m}}$$

* حسباً حاصل المقادير في عن ٣ حالات (Real, Zero, Imaginary) ما زالت المقادير $1 > 0$, $= 0$ و < 0

$$\boxed{1} \quad \frac{C^2}{2m} - \frac{k}{m} = 0, \quad (\text{Critically Damped})$$

$$S_0 \quad A_{1,2} = \frac{-c}{2m}$$

$$-\frac{C_c^2}{4m} - K = 0 \Rightarrow C_{cr}^2 = 4mK$$

$$\text{Critical Damping } (C_{cr}) \text{ gives } \zeta = 1 \leftarrow C_{cr} = 2 \sqrt{m k}$$

* Now we will define a new parameter $\{\underline{m}\}$

Damping Ratio ζ and $\zeta \leftarrow$

$$\{ = \frac{c}{C_{cr}} \text{ and dimensionless!}$$

Based on ? we have 11 Cases

if $C > C_{cr} \rightarrow \zeta > 1 \rightarrow$ Over damped

$$\zeta = \zeta_{cr} \longrightarrow \beta = 1 \rightarrow \text{critically damped}$$

$C < C_{cr} \rightarrow \beta < 1 \rightarrow$ under damped

$C = 0 \longrightarrow \zeta = 0 \rightarrow \text{Undamped}$

$$\rightarrow C = \{ \quad C_{cr} = \{ \times 2 * \sqrt{m k} \rightarrow \text{کافی یاری} \quad \sqrt{\frac{C^2}{4m} - 4w_n^2} = 0$$

$$\lambda_{1,2} = \pm \sqrt{\frac{m k}{m^2}} = \pm \sqrt{\frac{k}{m}}$$

$$\text{So} \quad \lambda_{1,2} = - \sqrt{\frac{k}{m}} \pm \sqrt{\frac{3^2 k}{m} - \frac{k}{m}}$$

$$= - \sqrt{w_n} \pm w_n \sqrt{3^2 - 1} \rightarrow \begin{array}{l} \text{وچایی} \\ \text{یکون ما را فل} \\ \text{اجز، مایساوی} \end{array}$$

So Based on this equation and the 4 cases of β :

1) For undamped Case: $\beta = 0, \lambda_{1,2} = \pm i w_n$

$$x(t) = A \cos(w_n t) + B \sin(w_n t)$$

2) For Underdamped Case: $0 < \beta < 1, \lambda_{1,2} = -\beta w_n \pm i w_n \sqrt{1 - \beta^2}$

here we define new parameter:

$$\text{Damping Frequency } (w_d) = w_n \sqrt{1 - \beta^2}$$

(damped natural frequency)

$$x(t) = A_1 e^{(-w_n + i w_d)t} + A_2 e^{(-w_n - i w_d)t} \quad \lambda_{1,2} = -w_n \pm i w_d$$

$$x(t) = e^{-\beta w_n t} (A_1 e^{i w_d t} + A_2 e^{-i w_d t}) \rightarrow$$

Using Euler's identity :

$$x(t) = e^{-j\omega_n t} (B_1 \cos(\omega_d t) + B_2 \sin(\omega_d t))$$

→ Recalling The Ic's → $x(0) = x_0$, $\dot{x}(0) = x_0$
 to find B_1 and B_2

$$\textcircled{1} \quad X_0 = B_1 \quad \xrightarrow{\text{wavy arrow}} \quad B_1 = X_0$$

$$\textcircled{2} \quad \dot{x}(t) = -j \omega_n e^{-j \omega_n t} (B_1 \cos(\omega_n t) + B_2 \sin(\omega_n t))$$

هون استھنھ مھارلہ

الله ∞ وأعُرف $\}$

$$\dot{X}_0 = V_0 = - \{ w_n \ B_1 + B_2 \ w_d \}$$

$$V_0 = - \{ w_n \} X_0 + B_2 w_0$$

$$B_2 = \frac{V_0 + 3 w_n x_0}{w_d}$$

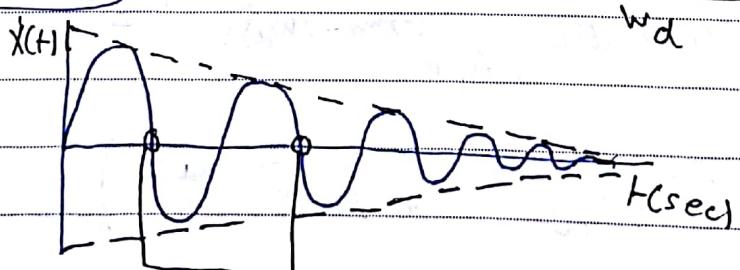
العزمية بالعمران

$$\Rightarrow x(t) = e^{-3\ln t} \left(x_0 \cos \omega_d t + \frac{V_0 + 3 \ln x_0}{\omega_d} \sin(\omega_d t) \right)$$

هاد الجزو
هو المسئول عن ا

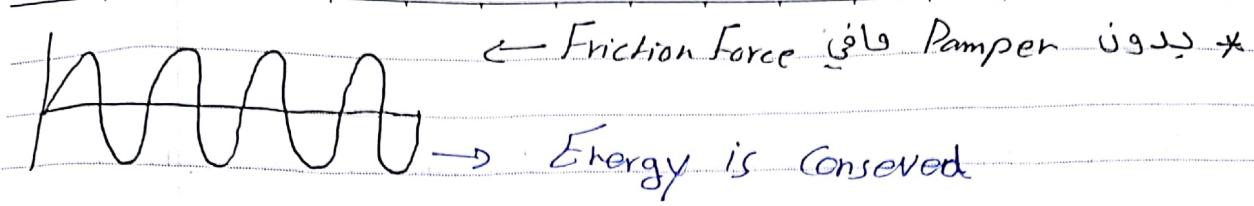
Decay

(Exponential
Decay)

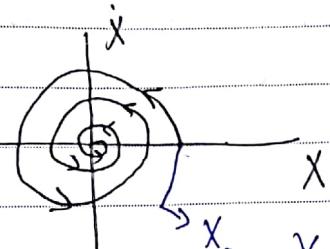


$$T_d \text{ (damped Period)} = \frac{2\pi}{\omega_d} = \frac{2\pi}{\omega_n \sqrt{1 - \zeta^2}}$$

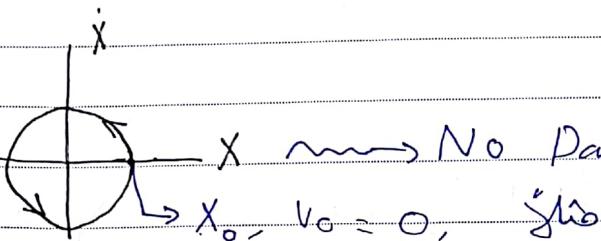
هاد تائیر اد Pamper



(Phase Plane Plot) \dot{X} vs X لو بدي ارسانی *



→ Damping (No Energy Conservation)



→ No Damping (Energy is Conserved)

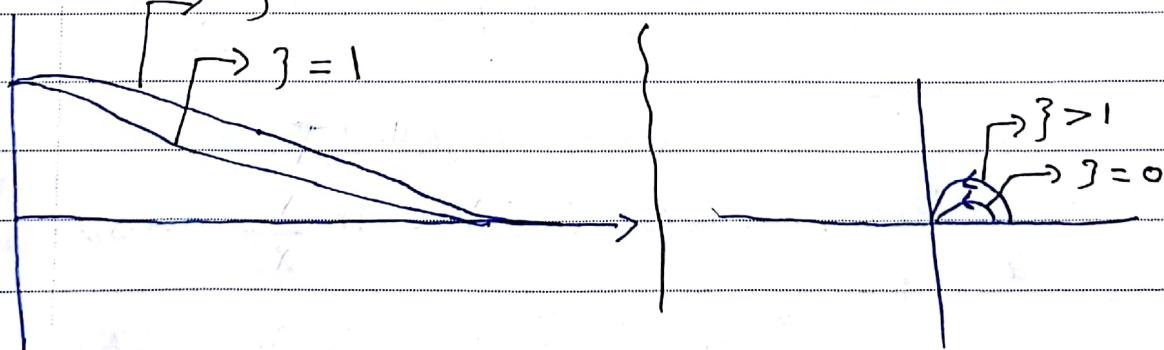
.. اذا كان في يعني برو للكسر ..
 ω_0, c, β معانى و اعلى
 $\therefore c=0, \beta=0$ و اذَا حل ثابتة ملائمة

Overdamped and

3) Critically damp Case ($\beta=1, \lambda_{1,2} = -\omega_n$)

(No vibration) !

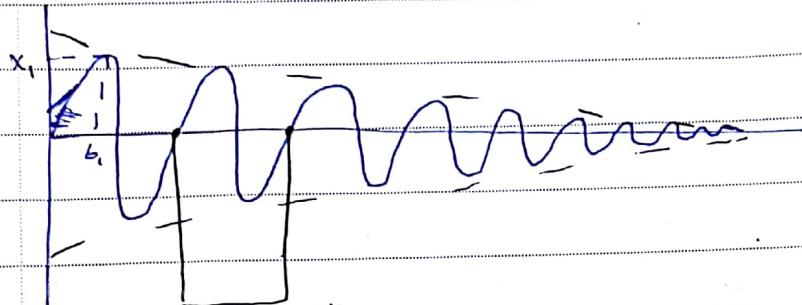
↳ repeated roots



* Logarithmic Decrement:

→ Used to find experimentally the amount of damping in the system.

→ Using a spring and a stopwatch



$$T_d = \frac{2\pi}{\omega_d}$$

lock up to x_1, t_1 with $\sin \omega t$

$$x(t) = e^{-3\omega_n t} (A \cos(\omega_d t + \phi)) \quad \rightarrow \sin \omega t$$

$$x_1 = e^{-3\omega_n t_1} (A \cos(\omega_d t_1 + \phi))$$

$$\boxed{t_2 = t_1 + T_d}$$

$$x_2 = e^{-3\omega_n t_2} (A \cos(\omega_d t_2 + \phi))$$

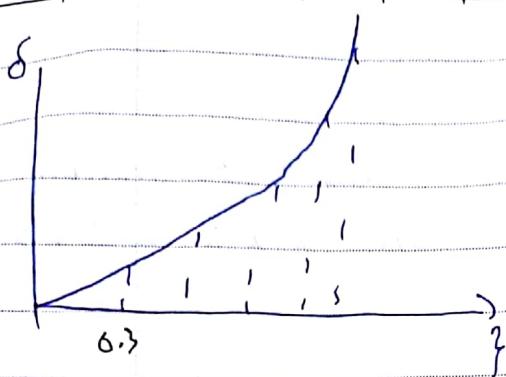
$$= e^{-3\omega_n t_1 - 3\omega_n T_d} (A \cos(\omega_d t_1) + 2\pi + \phi)$$

→ Define New Parameter (δ) = $\ln \frac{x_1}{x_2}$ \rightarrow logarithmic decrement

$$\delta = \ln e^{3\omega_n t_1} = 3\omega_n T_d = \frac{3\omega_n 2\pi}{\sqrt{1-\beta^2} \omega_n}$$

$$\delta = \ln \frac{x_1}{x_2} = \frac{2\pi\beta}{\sqrt{1-\beta^2}}$$

$$\delta = \frac{2\pi\beta}{\sqrt{1-\beta^2}}$$



→ For Lightly damped System $\beta \ll 1$

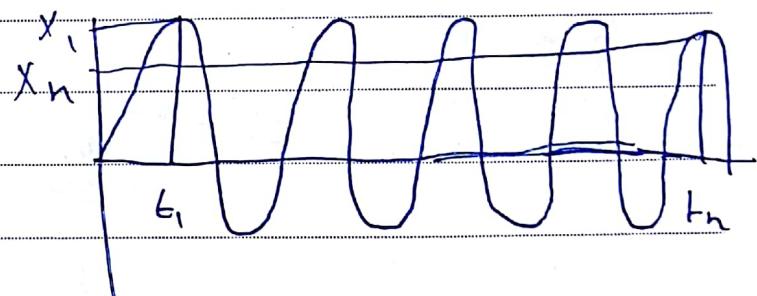
$$\ln \frac{x_1}{x_2} = \delta \approx 2\pi\beta \Rightarrow \beta = \frac{1}{2\pi} \ln \frac{x_1}{x_2}$$

→ For very Lightly Damped (أ يعني كثير وقليل تأثير لل damping)

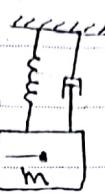
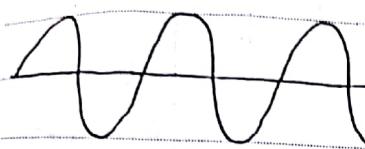
فهي تختلف في كل n cycles ~~فهي تختلف~~

$$\Rightarrow x_1, x_n \quad ! \quad x_1 \text{ يختلف عن } x_n$$

$$\Rightarrow t_n = t_1 + n T_d$$



Recall:

Static equilibrium
with x_0, \dot{x}_0 as I.C's

$$m\ddot{x} + Kx + C\dot{x} = 0 \quad \dots \quad (1)$$

$$\omega_n = \sqrt{\frac{K}{m}}, \quad C_{cr} = \sqrt{4Km}, \quad \zeta = \frac{C}{C_{cr}}$$

Cases:

① Undamped ($C=0, \zeta=0$)② Underdamped ($0 < \zeta < 1$)③ Critically damped ($\zeta=1$)④ Overdamped ($\zeta > 1$)

$$\delta = \ln \frac{x_1}{x_2} \Rightarrow \delta = 2\pi\zeta \quad (\text{Lightly damped})$$

divide eq ① over m to get

$$\ddot{x} + \frac{C}{m}\dot{x} + \frac{K}{m}x = 0, \quad$$

$$\frac{C}{m} = 2\zeta\omega_n \quad$$

$$\frac{K}{m} = \omega_n^2$$

$$\Rightarrow \ddot{x} + 2\zeta\omega_n\dot{x} + \omega_n^2x = 0 \quad \text{لجه ای این کام کوی خیلی سریع نمیشود}$$

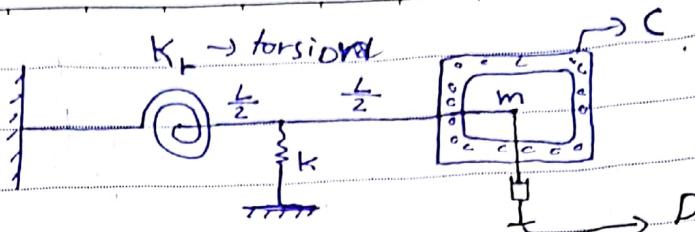
Generalized Undamped S.D.o.F System

ex: if the system Equation of Motion is $\ddot{q} + 3\dot{q} + 25q = 0$
we can find ω_n and ζ !

$$\rightarrow \omega_n^2 = 25 \Rightarrow \omega_n = 5 \quad \zeta = \frac{3}{10} \Rightarrow \text{Underdamped!}$$

(Underdamped!)

Example:



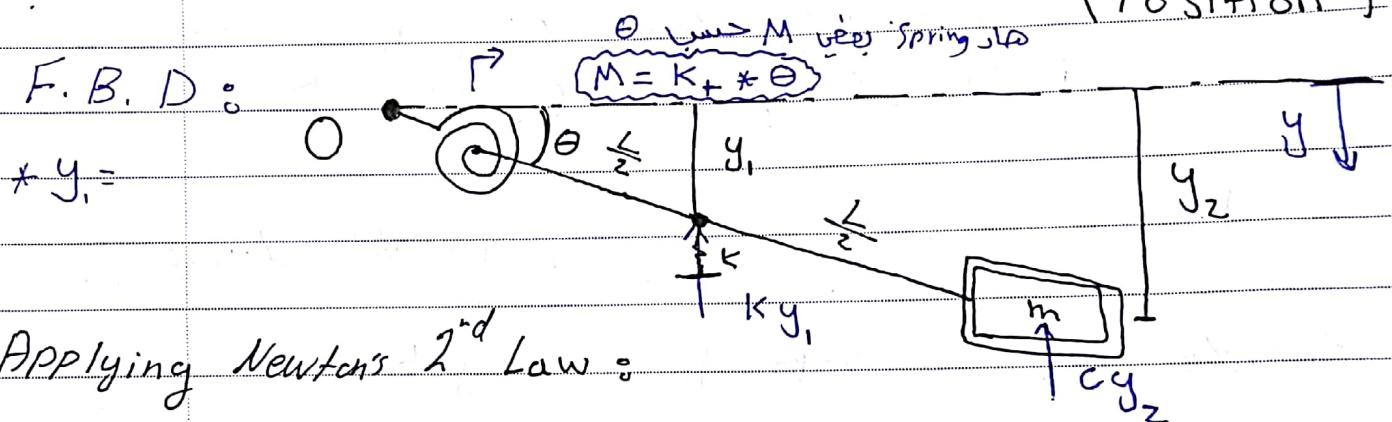
- Find: ① Equation of motion, ② C_{eq} , ③ K_{eq} , ④ M_{eq}
 ⑤ W_n , ⑥ ζ

- Showing: ① Applying Newton's 2nd Law

② Free body diagram

③ The Coordinate System

* the effect of $m g$ is cancelled by K_{dst} (static equilibrium position)



Applying Newton's 2nd Law:

$$\sum M_O = J_O \ddot{\theta} \quad * J_O: \text{mass moment of inertia}$$

$$ML^2 \ddot{\theta} = -K_T \theta - K_y_1 \frac{L}{2} \cos \theta \quad \rightarrow J_O = m L^2 \text{ as initial condition}$$

مسافة قوة about O

- $C y_2 L \cos \theta$!!. بـ

mass to rod

* For small θ , $\cos \theta \approx 1$, $\sin \theta \approx \theta$

so $y_1 = \frac{L}{2} * \theta$, $y_2 = L * \theta$

معدل الـ Moment of Inertia

القوة * المسافة يسوي الـ inertia

$$m L^2 \ddot{\theta} + K_T \theta + \frac{K L^2}{4} \theta + C L^2 \dot{\theta} = 0$$



$$m \frac{L^2}{m_{eq}} \ddot{\theta} + \left(K_e + \frac{KL^2}{4} \right) \theta + CL^2 \dot{\theta} = 0$$

No.

$m_{eq} = mL^2$

K_{eq}

C_{eq}

$$m_{eq} \ddot{y} + C_{eq} \dot{y} + K y = 0$$

$$\Rightarrow \omega_n^2 = \frac{1}{m_{eq}} \left(K_e + \frac{KL^2}{4} \right)$$

$$\omega_n = \sqrt{\frac{K_e}{m_{eq}} + \frac{KL^2}{4m_{eq}}}$$

ω_n & γ exist

$$\rightarrow 2\gamma \omega_n = \frac{CL^2}{m_{eq}} \Rightarrow \gamma = \frac{CL^2}{2\omega_n m_{eq}}$$

$$\Rightarrow \ddot{\theta} + 2\gamma \omega_n \dot{\theta} + \omega_n^2 \theta = 0 \quad I.C.'s = \theta(0), \dot{\theta}(0)$$

* Notice that we can write the equation

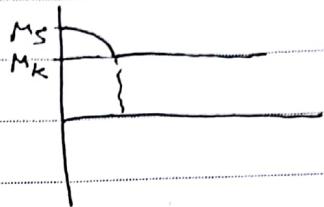
$$\text{in terms of } y_2. \quad \theta = \frac{y_2}{L}, \quad \dot{\theta} = \frac{\dot{y}_2}{L}, \quad \ddot{\theta} = \frac{\ddot{y}_2}{L}$$

$$\ddot{y}_2 + 2\gamma \omega_n \dot{y}_2 + \omega_n^2 y_2 = 0$$

* Notice that ω_n & γ are independent from the Coordinate System, and will always have the same value.

Dry friction Damping : "Coulomb damping"

M_s \rightarrow Static Coefficient of Friction



M_k \rightarrow Kinetic coefficient of Friction

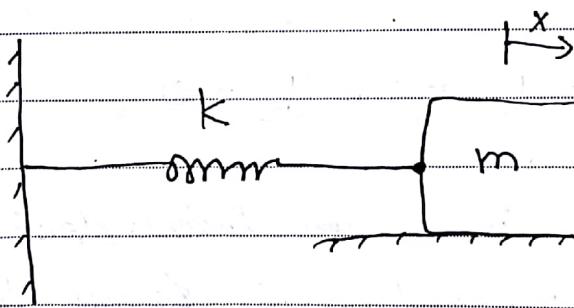
M depends on :

- 1) material properties of mating surfaces
- 2) Surface finish
- 3) Lubricants
- 4) Temperature

\rightarrow normal Force

$$F_{\text{Friction}} = M \times N$$

\hookrightarrow irreversible, non-conservative, produce heat



Contact means
there is

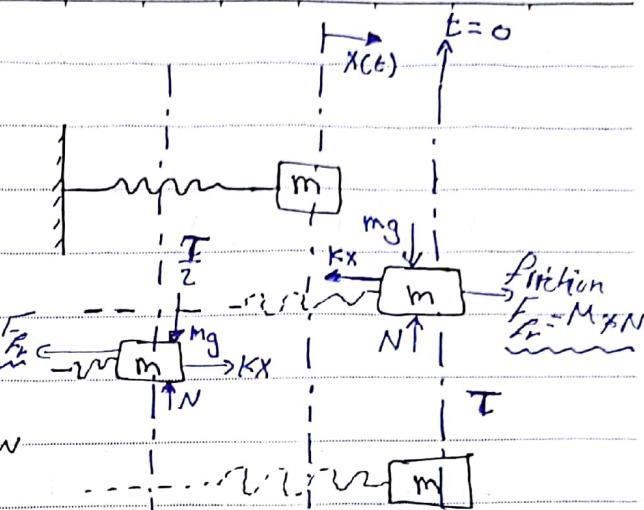
Friction Force

- Free Vibration :

- Dry Friction

$$- \text{Period} = \frac{2\pi}{\omega_n} = T$$

→ Applying Newton's 2nd law



at $0 \leq t \leq \frac{\pi}{\omega_n}$ (half cycle)

$$\sum F_x = m \ddot{x} \rightarrow m \ddot{x} + kx = M * mg$$

$$\Rightarrow \text{General Solution: } x_g = x_{c.s} + x_{p.i}$$

$$\left\{ \begin{array}{l} x(0) = x_0 \\ \dot{x}(0) = \text{constant} \\ = \text{zero} \end{array} \right.$$

$$\left\{ \begin{array}{l} x_{c.s} = A \cos(\omega_n t) + B \sin(\omega_n t) \end{array} \right.$$

$$\rightarrow x_{p.i} = \text{constant} \rightarrow k_c = M * mg$$

$$c = \frac{M * mg}{k}$$

$$x_{g.s} = A \cos(\omega_n t) + B \sin(\omega_n t) + \frac{M * mg}{k}$$

$$\rightarrow x(0) = x_0 = A + \frac{M * mg}{k} \rightarrow A = x_0 - \frac{M * mg}{k}$$

$$\rightarrow \dot{x}(0) = \dot{x}_0 = -A \sin(0) + B \cos(0) + 0 \rightarrow B = \dot{x}_0 = \text{zero}$$

$$\rightarrow x_{g.s}(t) = \left(x_0 - \frac{M * mg}{k} \right) \cos(\omega_n t) + \frac{M * mg}{k}$$

$$\rightarrow \dot{x}_{g.s}(t) = -\omega_n \times \left(x_0 - \frac{M * mg}{k} \right) \sin(\omega_n t) \quad \left\{ \begin{array}{l} 0 \leq t \leq \frac{\pi}{\omega_n} \end{array} \right.$$

Now we will find $X(\frac{\pi}{\omega_n})$ & $\dot{X}(\frac{\pi}{\omega_n})$ which will be our new IC for the 2nd half ($\frac{\pi}{\omega_n} \leq t \leq \frac{2\pi}{\omega_n}$)

$$\begin{aligned} X(\frac{\pi}{\omega_n}) &= \left(X_0 - \frac{Mmg}{K} \right) * (-1) + \frac{Mmg}{K} \\ &= -X_0 + \frac{2Mmg}{K} \end{aligned}$$

$\dot{X}(\frac{\pi}{\omega_n}) = \text{Zero!} \quad (V = \text{zero} \Rightarrow \text{Switch direction!})$

→ Apply Newton's 2nd law ($\frac{\pi}{\omega_n} \leq t \leq \frac{2\pi}{\omega_n}$)

$$\sum F = m \ddot{X} \rightarrow m \ddot{X} + KX = -Mmg$$

↳ negative sign

$$X_{g.s} = X_{c.s} + X_{p.I} \quad ! \text{ Due to } \text{Switch direction}$$

Same as before ← ↳ $X_{p.I} = C$

$$KC = -Mmg$$

$$\textcircled{a} \quad t = \frac{\pi}{\omega_n} \rightarrow X = -X_0 + \frac{2Mmg}{K} \quad \left. \begin{array}{l} C = -\frac{Mmg}{K} \\ \dot{X} = 0 \end{array} \right\} \text{I.C}$$

$$\Rightarrow X(\frac{\pi}{\omega_n}) = -X_0 + 2 \frac{Mmg}{K} = -A + 0 - \frac{Mmg}{K}$$

$$A = X_0 - \frac{2Mmg}{K} - \frac{Mmg}{K}$$

$$A = X_0 - \frac{3Mmg}{K}$$

$$\ddot{x}(t) = -A \omega_n \sin(\omega_n t) + B \omega_n \cos(\omega_n t)$$

$$\dot{x}\left(\frac{\pi}{\omega_n}\right) = 0, \quad \boxed{B=0}$$

$$\rightarrow x_{g.s} = \left(x_0 - \frac{3Mmg}{K}\right) \cos(\omega_n t) - \frac{Mmg}{K}$$

@ end of 2nd half Cycle ($\frac{2\pi}{\omega_n}$)

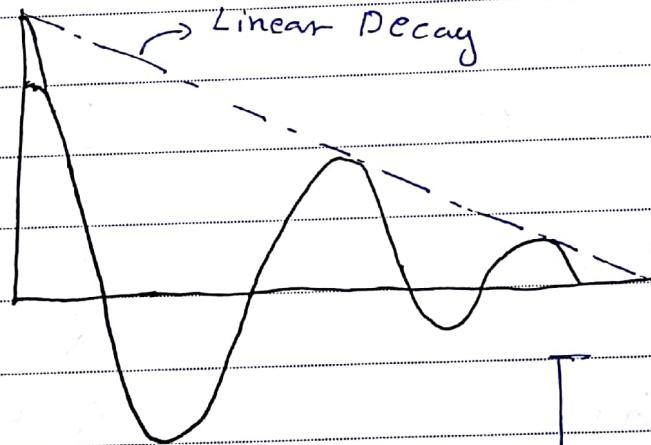
@ $\frac{\pi}{\omega_n} < t < \frac{2\pi}{\omega_n}$
Period

$$x\left(\frac{2\pi}{\omega_n}\right) = \left(x_0 - \frac{3Mmg}{K}\right) (1) - \frac{Mmg}{K}$$

$$= x_0 - 4 \frac{Mmg}{K} !$$

لما نجي الدور $\frac{2\pi}{\omega_n}$ دايم ما نجي دايم x في Cycle (في كل دورة)

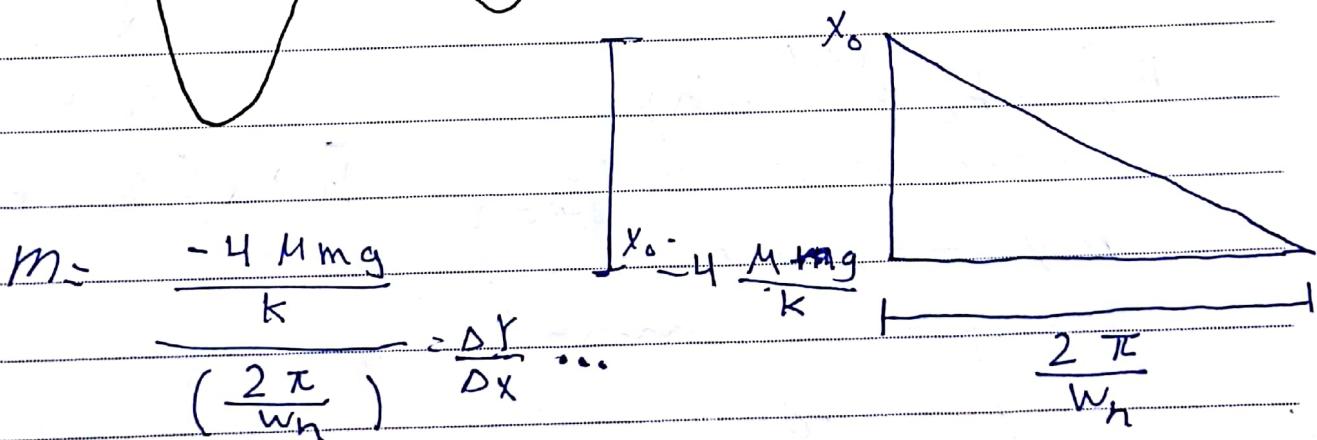
Linear Decay is $\frac{2Mmg}{K}$ دعبي دعبي



* Dry friction

doesn't effect

the natural frequency



Chapter 3 : Forced Vibration of SDOF "Harmonic" Excitation

$$F(t) = F_0 \sin(\omega t) \quad \text{أعلى قوّة جي} \leftrightarrow$$

Amplitude \downarrow ω excitation frequency (rad/s)

$$\omega = \frac{2\pi}{T}$$

$$= F_0 \cos(\omega t) \quad \underbrace{\sin, \cos, e^{j\omega t}}_{\text{Harmonic forces مفعول}} \downarrow$$

$$= F_0 e^{j\omega t} \quad \text{Harmonic forces مفعول}$$

* Undamped System ($c=0, \zeta=0$) : جيفرن منفع

Actuator ①

Example : Rotating ②

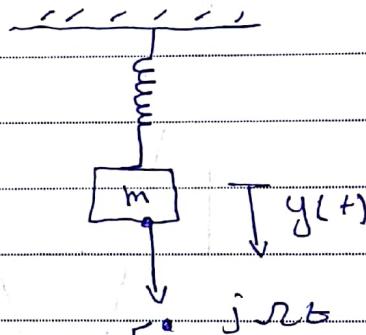
a spring with static position
under Harmonic excitation : Unbalanced
Machine

① First we find the
Equation of motion

→ Newton's 2nd law :

$$my'' = F_0 e^{j\omega t} - Ky$$

$$my'' + Ky = F_0 e^{j\omega t}$$



$y(0)$ & $y'(0)$ are IC's

↳ 2nd order ODE BUT non-homogeneous

as Previous Solution..

$$y_{g.s} = y_{c.s} + y_{p.i}$$

$$y_{c.s} = A \cos(\omega_n t) + B \sin(\omega_n t)$$

→ Linear System Output follows input..

So we assume:

$$y_{p.i} = Y * e^{j\omega_n t}$$

$$\dot{y}_{p.i} = Y j \omega_n e^{j\omega_n t}$$

$$\ddot{y}_{p.i} = -Y \omega_n^2 e^{j\omega_n t}$$

$$m(-Y \omega_n^2 e^{j\omega_n t}) + K(Y e^{j\omega_n t}) = F_0 e^{j\omega_n t}$$

Substitute in f.o.m

$$(K - m \omega_n^2) Y e^{j\omega_n t} = F_0 e^{j\omega_n t}$$

$$Y = \frac{F_0}{K - m \omega_n^2}$$

مدولار م
من نفس

العلـى

فـالـجـمـعـ

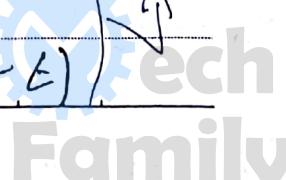
نفسـ

الـحـلـ

$$x_{p.i} = \frac{F_0}{K - m \omega_n^2} * e^{j\omega_n t}$$

$$= \frac{F_0}{K - m \omega_n^2} * \sin(\omega_n t)$$

$$= \frac{F_0}{K - m \omega_n^2} * \cos(\omega_n t)$$



Let's Study

$$Y = \frac{F_0}{K - m \omega^2}$$

→ devide by K

$$\omega_n^2 = \frac{K}{m}$$

$$Y = \frac{F_0 / K}{1 - \frac{(m \omega^2)}{(K \omega_n^2)}}$$

$$Y = \frac{Y_{\text{static}} \quad 1}{1 - \frac{\omega^2}{\omega_n^2} \quad 2}$$

$$Y = \frac{Y_{\text{static}}}{1 - \frac{\omega^2}{\omega_n^2}}$$

1

F_0 / K : Static deflection

$$Y_{\text{static}} = \frac{F_0}{K}$$

2

$$Y = \frac{\omega}{\omega_n}$$

frequency ratio

the ratio of

external excitation

frequency to the system

natural frequency

$$\frac{Y}{Y_{\text{static}}}$$

⇒ Magnification factor

↳ draw r vs MF :

$$\frac{Y}{Y_{\text{static}}}$$

Resonance (عند $r=1$)
 MF goes to ∞ and system failure
 break down!

* ممكناً في التصميم
 يكون بعيد عن ممكناً

$$Y_{\text{st}} = \frac{1}{1 - r^2}$$

$$r = 1$$

⇒ Resonance

Original Equipment Manufacturer (OEM) and How to Pass

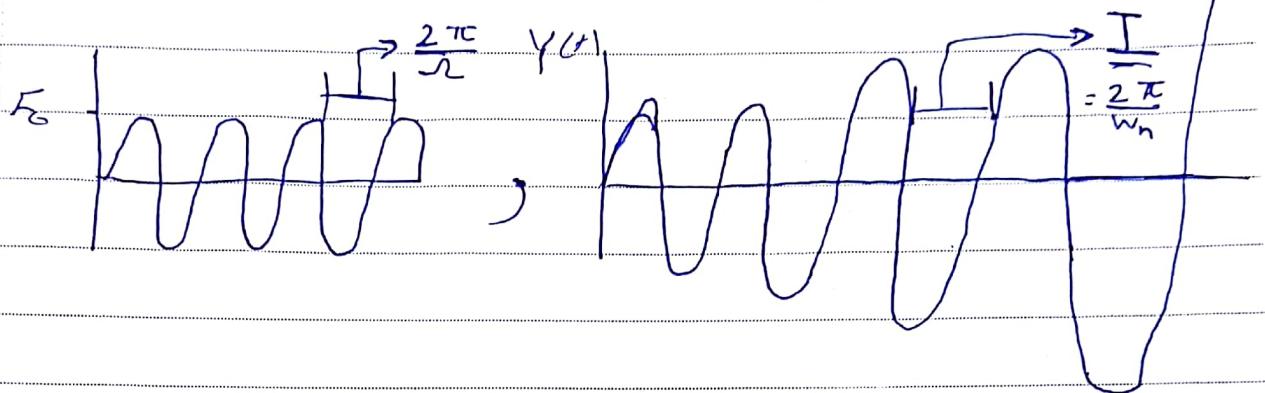
Critical Speed

Rotating speed in machine

$$r = \frac{\omega}{\omega_n}$$

Provide 5 critical speed

Resonance occurs when $\omega_n = \omega$ (Same phase)



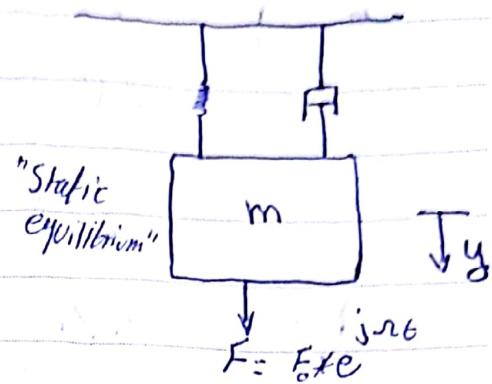
الآن همیشہ بیلش کبھی و بدوغ الی ∞

For underdamped ($0 < \zeta < 1$)

Equation of motion:

$$m\ddot{y} + c\dot{y} + Ky = F_0 e^{j\omega t}$$

while $y(0)$ & $\dot{y}(0)$ are IC's



$$y_{g.s} = y_{c.s} + y_{p_i}$$

* for Linear System \rightarrow Output follows the input BUT with Lag!

For the Particular Solution $\rightarrow Y_{p_i} = Y \cdot e^{j\omega t}$

$$\dot{Y}_{p_i} = Y j \omega e^{j\omega t}$$

$$\ddot{Y}_{p_i} = -Y \omega^2 e^{j\omega t}$$

Substitute in EoM

$$-m Y \omega^2 e^{j\omega t} + c Y j \omega e^{j\omega t} + K Y e^{j\omega t} = F_0 e^{j\omega t}$$

$$\Rightarrow Y = \frac{F_0}{K - m \omega^2 + c j \omega}$$

$$a + jb = \sqrt{a^2 + b^2} e^{j\alpha}$$

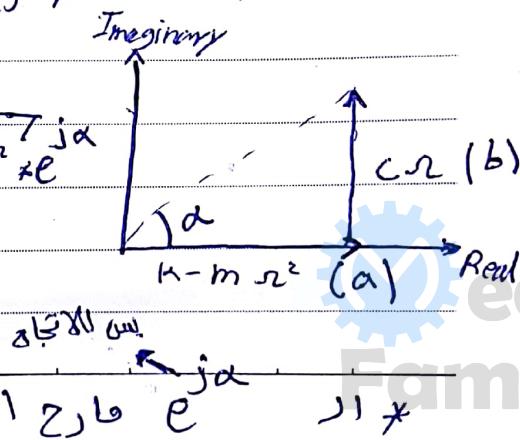
Complex Dynamic Stiffness

$$\alpha = \tan^{-1}\left(\frac{b}{a}\right)$$

$$\text{So } \Rightarrow K - m \omega^2 + c \omega j = \sqrt{(K - m \omega^2)^2 + (c \omega)^2} e^{j\alpha}$$

$$\begin{bmatrix} a \\ b \end{bmatrix}$$

$$\text{and } \alpha = \tan^{-1}\left(\frac{c \omega}{K - m \omega^2}\right)$$



$$e^{j\alpha} \text{ طرح المفعول لأنها أثنا بحسب الفيزياء}$$

$$y = \frac{F_0 e^{j\omega t}}{\sqrt{(k-m\omega^2) + (c\omega)^2}} \rightarrow \text{divide by } (k)$$



$$y = \frac{F_0 / k}{\sqrt{\left(1 - \frac{\omega^2}{\omega_n^2}\right)^2 + (2\zeta\omega)^2}} \rightarrow y_{\text{static}}$$

$$\text{and } r = \frac{\omega}{\omega_n}$$

$$\frac{y}{y_{\text{sf}}} = \frac{1}{\sqrt{\left(1 - \frac{\omega^2}{\omega_n^2}\right)^2 + (2\zeta\omega)^2}} = M.F$$

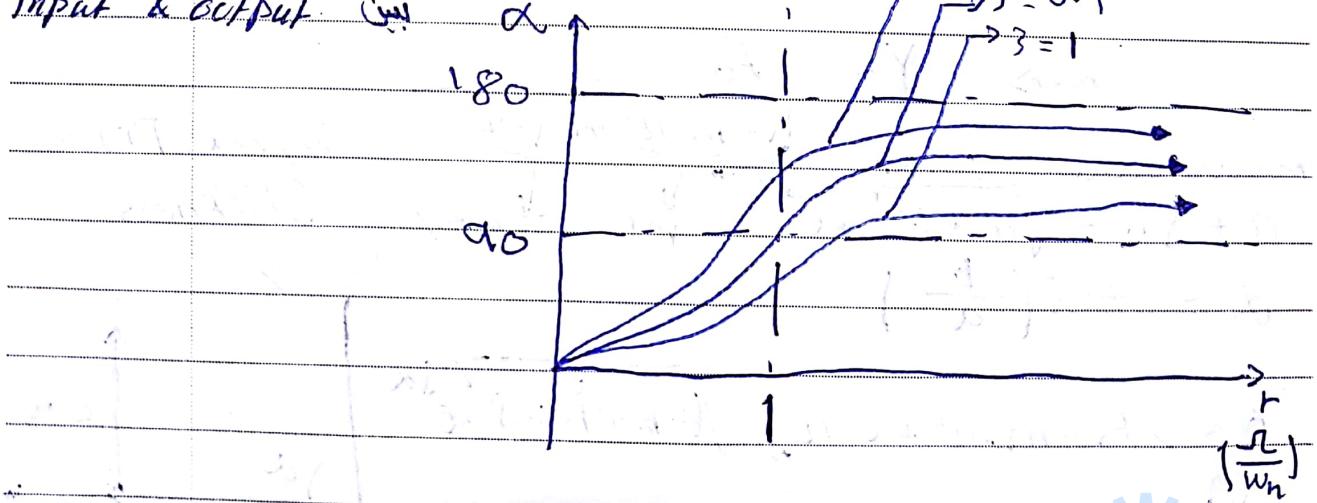
$$|y_r| = M.F$$

(near resonance) \leftarrow

angle α

Phase Shift \rightarrow

input & output \rightarrow α



Plot the Curve :

X-axis $\rightarrow r$

$r = (0.5 - 5)$, increment (0.1)

For $\beta = 0.1, 0.8, 1, 1.2$

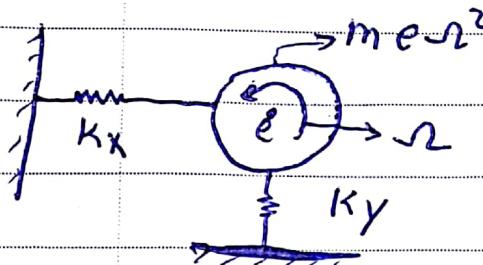
Y-axis $\rightarrow \frac{y}{y_{st}}$

$$y_{c.s} = e^{-\beta \omega_n t} (A \cos(\omega_n t) + B \sin(\omega_n t))$$

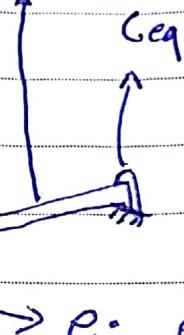
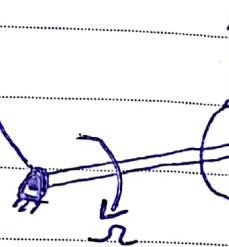
$$y_{p.i} = \frac{F_0/k}{\sqrt{(1-r^2) + (2\beta r)^2}} * e^{-\beta \omega_n t}$$

Shaft
Stiffness

* Tefcot rotor :



Centrifugal = $m\omega_r^2$



C_{eq}

C_{eq}

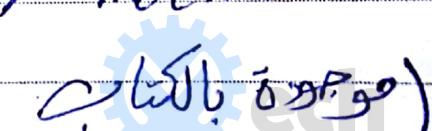
e : eccentricity
"difference between
mass center and
the geometrically
rotation center"

Home work:

1 Plot (the above)

2 find the equation of motion

for Tefcot rotor

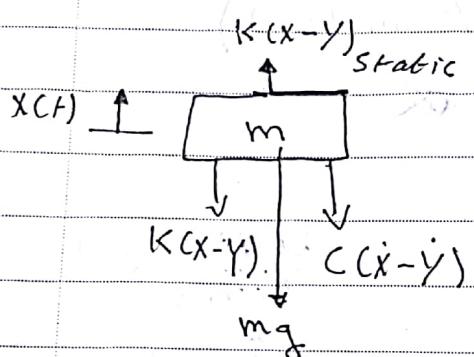


Response of SDOF System under ^{Base} harmonic excitation

The equation of motion

Free body diagram

assuming $x > y$



Road irregularity
 $y = Y e^{j\omega t}$

* Applying Newton's 2nd law:

$$\sum F_x = m\ddot{x} = -K(x-y) - C(\dot{x}-\dot{y})$$

$$\Rightarrow m\ddot{x} + C\dot{x} + Kx = C\dot{y} + Ky \quad \text{Known displacement}$$

x, \dot{x} @ Zero are our Initial Conditions

$$y = Y e^{j\omega t}, \dot{y} = Y j\omega e^{j\omega t}, \ddot{y} = -Y\omega^2 e^{j\omega t}$$

Substitute in the original Equation:

$$m\ddot{x} + C\dot{x} + Kx = C(Y j\omega e^{j\omega t}) + K(Y e^{j\omega t})$$

So we have Linear System Output that is following the input.

$$x_{g.s.} = x_{c.s.} + x_{p.i.}$$

For the Particular Integral ($x_{p.i.}$)

$$x = X e^{j\omega t} \rightarrow \dot{x} = X j\omega e^{j\omega t} \rightarrow \ddot{x} = -X\omega^2 e^{j\omega t}$$

Substitute in the original Equation:

$$(K - m\omega^2)x e^{j\omega t} + jC\omega x e^{j\omega t} = (K + jC\omega)Y e^{j\omega t}$$

Output \rightarrow

$$\frac{x}{Y} = \frac{K + jC\omega}{(K - m\omega^2) + jC\omega}$$

Dimensionless

$$\text{and, } K + jC\omega = \sqrt{K^2 + C^2\omega^2} e^{j\phi}$$

Leaving

Displacement Transmissibility Ratio

$$\phi = \tan^{-1} \left(\frac{C\omega}{K} \right)$$

$$\text{and } (K - m\omega^2) + jC\omega = \sqrt{(K - m\omega^2)^2 + C^2\omega^2} e^{j\beta}$$

$$\frac{x}{Y} = \sqrt{1 + (2\beta r)^2}$$

$$\beta = \tan^{-1} \left(\frac{C\omega}{(K - m\omega^2)} \right)$$

$$\sqrt{(1 - r^2)^2 + (2\beta r)^2}$$

divide by r to get

$$r = \frac{\omega}{\omega_n}, \beta = \frac{C}{C_{cr}}$$

$C=0$ \leftarrow no Damping (سيؤدي إلى أن المعادلة تبقي كما هي)

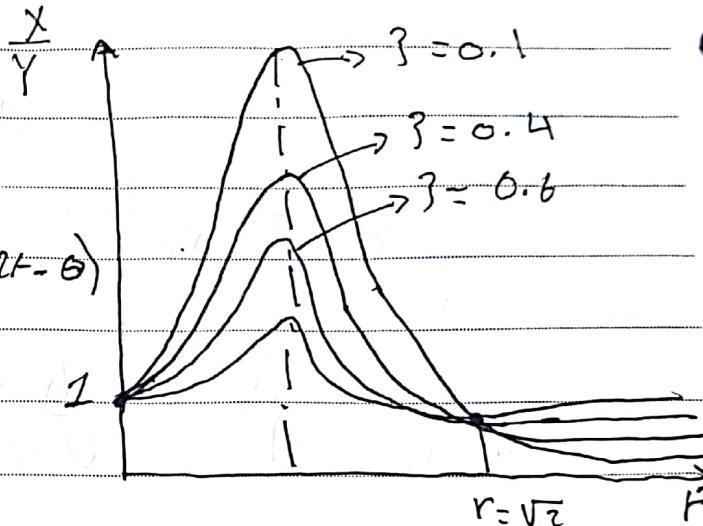
$$\frac{X}{Y} = \frac{1}{1-r^2}$$

تبقي المعادلة

* to find The Maximum Value we derivate $\left(\frac{d(X)}{dr} \right)$

$$X_p(t) = \frac{Y \sqrt{1 + (2\beta r)^2}}{\sqrt{(1-r^2)^2 + (2\beta r)^2}} e^{j(\omega t - \theta)}$$

$$X_p(t) = \frac{Y \sqrt{1 + (2\beta r)^2}}{\sqrt{(1-r^2)^2 + (2\beta r)^2}} * \sin(\omega t - \theta)$$

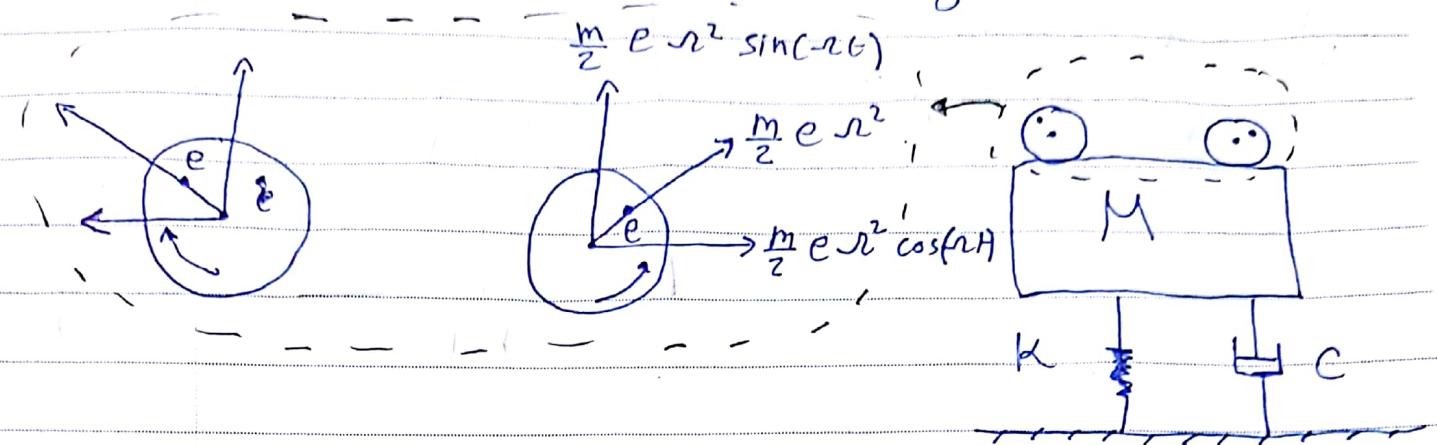


$$X_{g-s} = e^{-j\omega_n t} (A \cos \omega_n t + B \sin \omega_n t) +$$

$$+ \frac{Y \sqrt{1 + (2\beta r)^2}}{\sqrt{(1-r^2)^2 + (2\beta r)^2}} * \sin(\omega t - \theta)$$

\rightarrow Apply The I.C's to find A and B.

Response of SDOF due to rotating Unbalance:

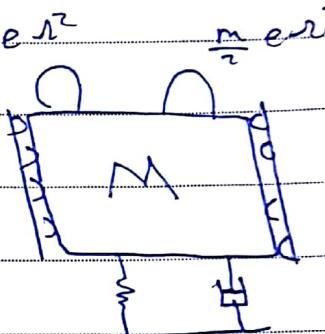


Center of mass \rightarrow و مي بع دل هي لـ e

Center of rotation \rightarrow عن دل

Vibration دل السبب الليئسي

For a SDOF \rightarrow و أمد اذوا كانون نفس يعني فالحملة
لليعن و اليسار بلغا يعني!



\Rightarrow Replace $F_0 = me r^2$

$$X(t) = X_0 e^{j \omega t}, \quad F(t) = me r^2 e^{j \omega t}$$

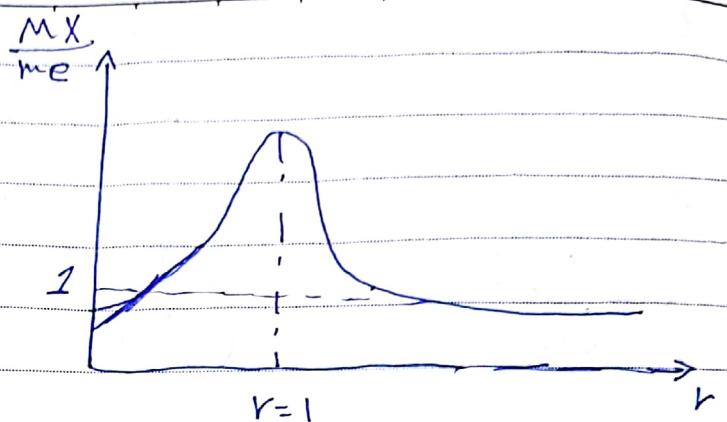
$X = \frac{F_0}{\sqrt{(k - M \omega^2)^2 + (c \omega)^2}}$ if we divide by K

$$\text{and } r = \omega, \beta = \frac{c}{M \omega}, K = M \omega^2$$

$$\frac{\omega^2}{\omega_n^2} = M$$

$$\frac{M X}{me} = \frac{\omega^2}{\sqrt{(1 - r^2)^2 + (2 \beta r)^2}}$$

$$X = \sqrt{1 + (23r)} \\ \sqrt{(r-r^2)^2 + (23)^2}$$



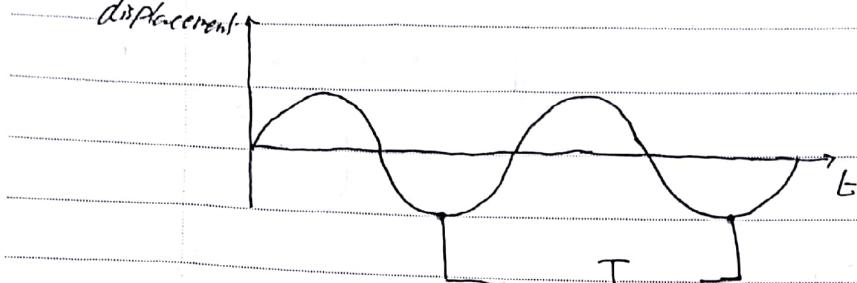
End of
first exam

material

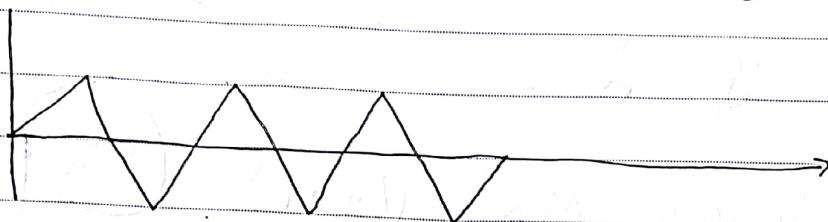
Revision lecture

* Vibration: Dynamic Motion that repeats it Self.

displacement



↳ Period (seconds) (time) to
Complete one Cycle



Frequency (f): No. of cycle completed in One Second (CPS)
(Hz)

Mathematically: $\omega = 2\pi f = \frac{2\pi}{T}$ (rad/s)

* $f = \text{rps} = \frac{\text{rpm}}{60}$!

* Classification $\begin{cases} \xrightarrow{\text{Free Vibration}} \text{Mathematical Model} \\ \xrightarrow{\text{Forced Vibration}} \text{by Physical rules (Newton)} \end{cases}$

No. of independent
Position Co-ordinate.

Applied to each

D.O.F.

F.B.D.
with
Assumption

for Free vibration: $x(t), y(t), \theta(t), \varphi(t)$

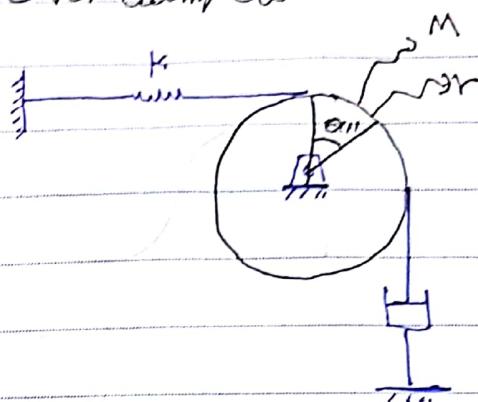
Classification: - Undamped - Under damped
- Critically damped - Over damped

Assumptions:

① Small motions

② Linear spring

③ viscous damper

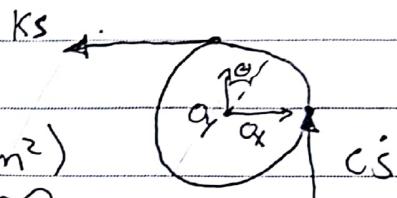


applying Newton's 2nd law:

* The F.B.D is drawn with deflections

$$\sum M_o = J_o \ddot{\theta}$$

$$J_o \rightarrow \text{For disk } \frac{1}{2} M R^2 \quad (\text{Kg} \cdot \text{m}^2)$$



$$\frac{1}{2} M R^2 \ddot{\theta} = -K s r - C s \dot{r}$$

$$s = r \theta$$

replace s by θ

$$\dot{s} = r \dot{\theta}$$

$$\Rightarrow \frac{1}{2} M R^2 \ddot{\theta} + K r^2 \theta + C r^2 \dot{\theta} = 0$$

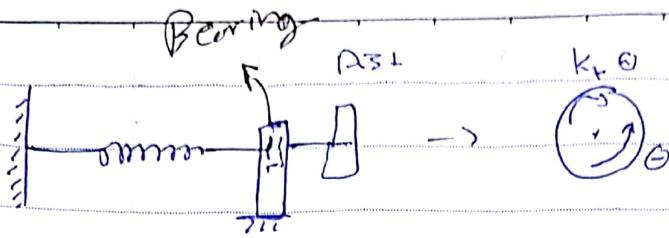
$$M \ddot{\theta} + 2C \dot{\theta} + 2K \theta = 0 \rightarrow \text{divide by } M$$

23^{Wn}

$$\ddot{\theta} + \frac{2C}{M} \dot{\theta} + \frac{2K}{M} \theta = 0$$

$$\omega_n = \sqrt{\frac{2K}{M}} \quad (\text{rad/s})$$

$$\zeta = \frac{C}{M \omega_n}$$



$$\sum M_o = J_o \ddot{\theta} + \cancel{M}$$

$$J_o \ddot{\theta} + k_f \theta = 0$$

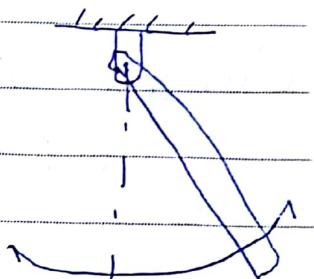
$$\ddot{\theta} + \frac{k_f}{J_o} \theta = 0$$

$$\omega_n^2 = \frac{k_f}{J_o}$$

$$\omega_n = \sqrt{\frac{k_f}{J_o}}$$

For Compound Pendulum (True Normal Pendulum)

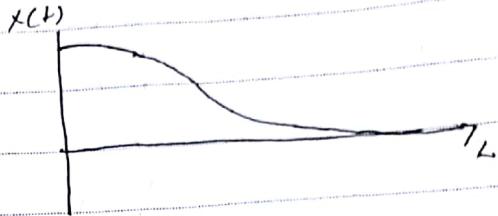
$$\omega_n = \sqrt{\frac{k_f}{J_o}}$$



نَكَلَةِ اسْتِقْرَأَةِ الْمَادَدَاتِ لَا يَمْلِئُ حَالَتِنِ

$$\lambda_{1,2} = (-\beta \pm \sqrt{\beta^2 - 1}) \omega_n$$

$$= -\omega_n$$



$$\Rightarrow x(t) = C_1 e^{-\omega_n t} + C_2 t e^{-\omega_n t}$$

$$= e^{-\omega_n t} (C_1 + C_2 t)$$

$$\text{if } x(0) = x_0, \dot{x}(0) = \dot{x}_0 \Rightarrow C_1 = x_0, C_2 = \dot{x}_0 + x_0 \omega_n$$

$$x(t) = (x_0 + (\dot{x}_0 + x_0 \omega_n) t) e^{-\omega_n t}$$

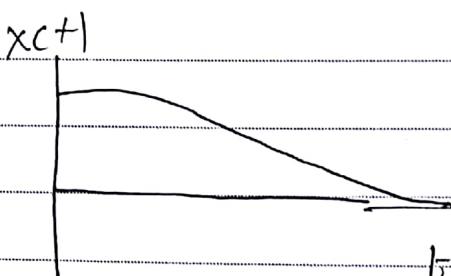
Over damped $\sqrt{\beta^2 - 1} \rightarrow \text{Positive}$

$$\begin{aligned} \lambda_1 &= (-\beta + \sqrt{\beta^2 - 1}) \omega_n \\ \lambda_2 &= (-\beta - \sqrt{\beta^2 - 1}) \omega_n \end{aligned} \quad \left. \begin{aligned} &\text{Both will always} \\ &\text{be negative} \end{aligned} \right\}$$

$$x(t) = C_1 e^{(-\beta + \sqrt{\beta^2 - 1}) \omega_n t} + C_2 e^{(-\beta - \sqrt{\beta^2 - 1}) \omega_n t}$$

$$\text{for } x(0) = x_0, \dot{x}(0) = \dot{x}_0 \Rightarrow$$

$$C_1 = \frac{x_0 \omega_n (\beta + \sqrt{\beta^2 - 1}) + \dot{x}_0}{2 \omega_n \sqrt{\beta^2 - 1}}$$

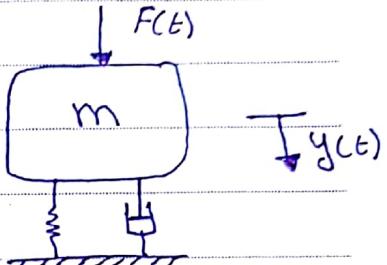
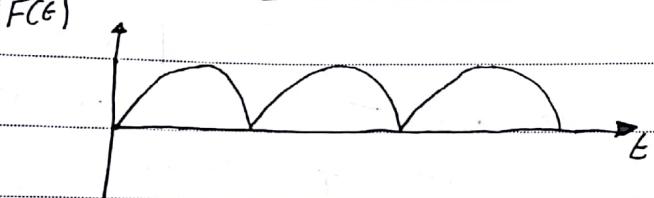
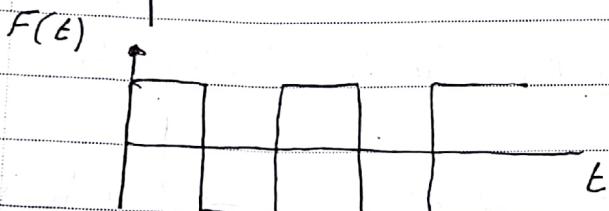
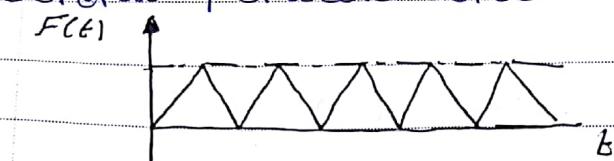


$$C_2 = \frac{-x_0 \omega_n (\beta - \sqrt{\beta^2 - 1}) - \dot{x}_0}{2 \omega_n \sqrt{\beta^2 - 1}}$$

Vibration

Chapter 4: Response under General loading

General periodic force



* Any periodic function can be represented as a series summation using Fourier Series.

$$F(t) = \frac{a_0}{2} + \sum_{i=1}^{\infty} a_i \cos(i\omega t) + b_i \sin(i\omega t)$$

where $\omega = \frac{2\pi}{T} = 2\pi f$

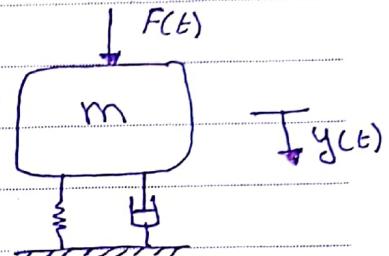
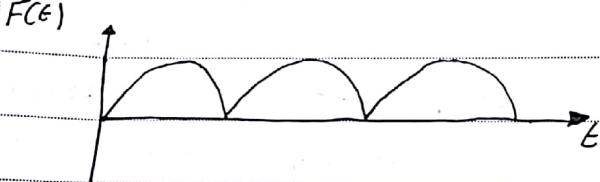
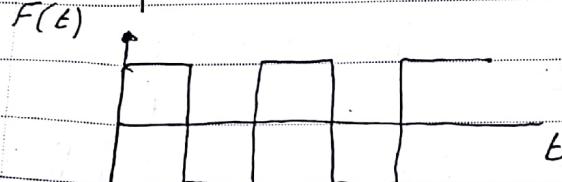
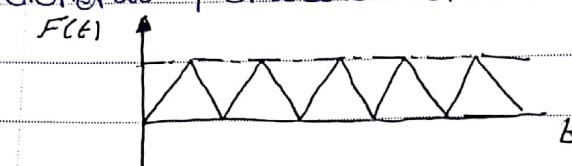
Fundamental frequency

Vibration

Chapter 4: Response under General loading



General periodic force



* Any periodic function can be represented as a series summation using Fourier Series.

$$F(t) = \frac{a_0}{2} + \sum_{i=1}^{\infty} a_i \cos(i\omega t) + b_i \sin(i\omega t)$$

where $\omega = \frac{2\pi}{T} = 2\pi f$

Fundamental frequency

Multiply both sides of the equation by $\cos(i\omega t)$ and integrate over one period

$$\int F(t) \cos(i\omega t) dt = \int_0^T \frac{a_0}{2} \cos(i\omega t) dt$$

$$+ \sum \int_0^T a_i \cos^2(i\omega t) dt$$

$0 \rightarrow \infty$

$$+ \sum \int_0^T b_i \sin(i\omega t) \cos(i\omega t) dt$$

$$* a_i = \frac{2}{T} \int_0^T F(t) \cos(i\omega t) dt$$

$$* b_i = \frac{2}{T} \int_0^T F(t) \sin(i\omega t) dt$$

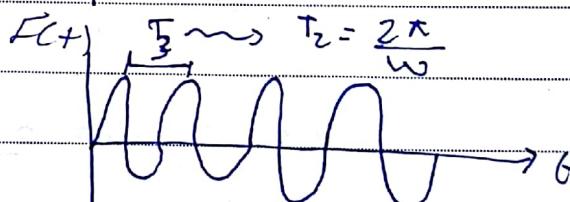
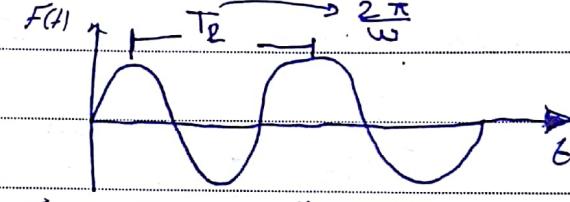
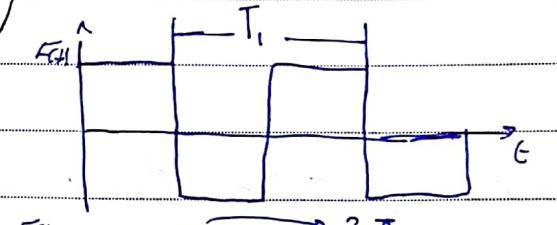
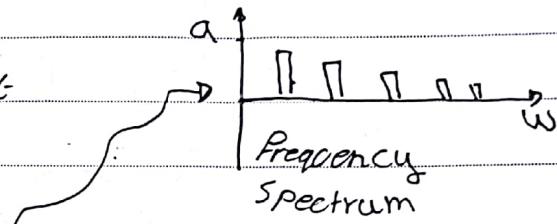
The Fourier Coefficients:

$$+ m\ddot{y} + c\dot{y} + Ky = F(t)$$

$$\Rightarrow m\ddot{y} + c\dot{y} + Ky = \frac{a_0}{2} + \sum_{i=1}^{\infty} a_i \cos(i\omega t)$$

$$+ \sum_{i=1}^{\infty} b_i \sin(i\omega t)$$

after solving the O.D.E for the given forcing function $y_{g.s} = y_{c.s} + y_{p.I}$

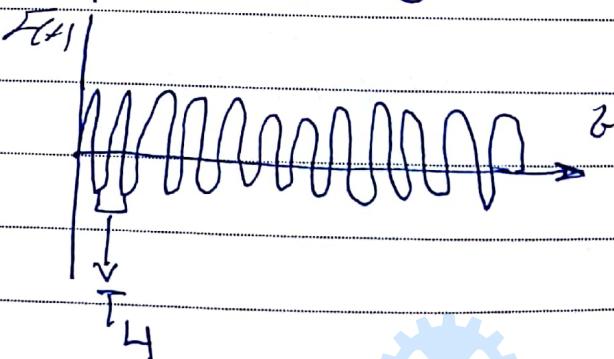


Since we are dealing with linear system the method of superposition

applies, then: $m\ddot{y}_i + c\dot{y}_i + Ky_i = a_i \cos(i\omega t)$

$$m\ddot{y}_i + c\dot{y}_i + Ky_i = b_i \sin(i\omega t)$$

$$m\ddot{y}_i + c\dot{y}_i + Ky_i = \frac{a_0}{2}$$



↳ See next Page for the General Eq.

* Harmonic motion:

$y(t) = \sum_{i=1}^{\infty} y_i(t)$, for all the harmonics.

the particular Integral Solution is:

$$y_{PI} = \sum_{i=0}^{\infty} \frac{i A_i / k \cos(i \omega t + \phi_i)}{\sqrt{(1 - (i \nu)^2)^2 + (2 \beta i \nu)^2}}$$

$$+ \frac{i B_i / k \sin(i \omega t + \phi_i)}{\sqrt{(1 - (i \nu)^2)^2 + (2 \beta i \nu)^2}}$$

* Resonance:

$$\begin{aligned} \nu = \omega_n &\Rightarrow \nu = 1 \\ i \nu = \omega_n &\Rightarrow \nu = i \end{aligned} \quad \left. \begin{array}{l} \\ \end{array} \right\} \quad \text{2 Points!}$$

→ Possibilities of resonance increase!

the general equation for previous page

$$y_i(t) = \frac{a_i / k}{\sqrt{(1 - (i \nu)^2)^2 + (2 \beta i \nu)^2}}, \cos(i \omega t - \phi)$$

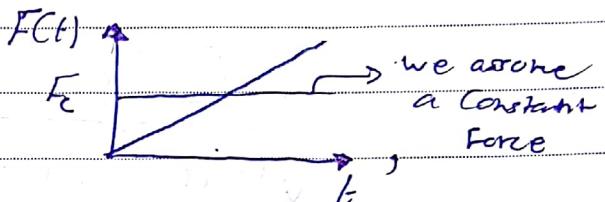
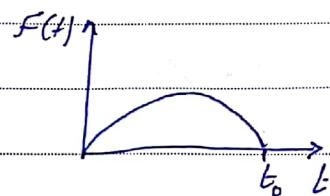
So if the General loading is :

1 Periodic \rightarrow Fourier analysis \rightarrow Cos & Sine
 \rightarrow exponential form

2 Non-Periodic input :

- doesn't repeat itself, act for sometime and disappears.

example :

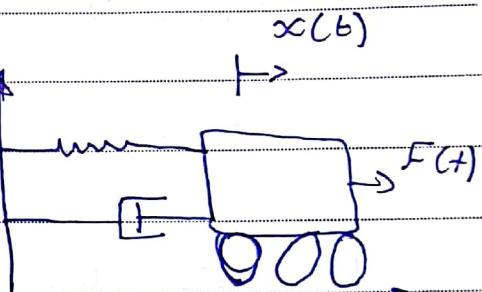


so for this system :

E.O.M. :

$$m\ddot{x} + c\dot{x} + kx = F_c$$

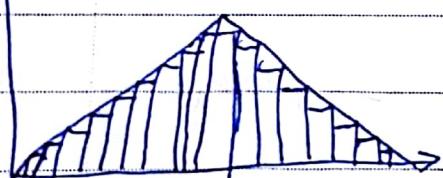
$F(t) \rightarrow$



2nd order O.D.E!

Force \rightarrow قیمتی کیمی کیمی

Constant Force... is equal to loading



Superposition

(Superposition) \rightarrow مکانیکی

input \rightarrow Output \rightarrow مکانیکی Output \rightarrow

So we get :

- 1] Response to each Pulse
- 2] Summation
- 3] Duhamel's Integral (Convolution)

* Alternative methods: ① Laplace transformation
② Numerical integration

* What actually happens in Laplace domain we freeze the time and study the behavior in S-domain ; S is (iw) .

* it's more like conversion

* Laplace Function Conditions:

* anything physical can be expressed in Laplace!

* it converts from differential to polynomial expressions.

* Valid method for Solving Linear System differential equation

- * It takes care of the initial condition automatically
- * To go to time domain we do inverse Laplace transformation.

Check Laplace transformation table for solving

Section 4.7 \rightarrow Very important!

The SDOF System:

$$m\ddot{x} + c\dot{x} + kx = f(t)$$

Taking the Laplace

$$L(m\ddot{x} + c\dot{x} + kx) = L(f(t))$$

* For $I^c = \text{zero}$ {

No.

Recall ~

$$X(t) = \int_0^\infty x(t) e^{-st} dt$$

Linear ODE

Our equation is $m \frac{d^2x}{dt^2} + c \frac{dx}{dt} + kx = f(t)$

$$\mathcal{L}(x(t)) = X(s)$$

For $x \rightarrow \mathcal{L}\left(\frac{dx}{dt}\right) = \int_0^\infty \frac{dx}{dt} e^{-st} dt$

$$u = e^{-st} \rightarrow du = -se^{-st}$$

$$dv = \frac{dx}{dt} \rightarrow v = x(t)$$

$$\mathcal{L}\left(\frac{dx}{dt}\right) = sX(s) - x(0) \rightarrow \text{Initial Condition}$$

$$\mathcal{L}\left(\frac{d^2x}{dt^2}\right) = s^2 X(s) - s x(0) - \dot{x}(0)$$

* From Laplace Transform table $\rightarrow \mathcal{L}(c) = \frac{C}{s}$ Step input
 $\mathcal{L}(ct) = \frac{C}{s^2}$

$$\mathcal{L}(m \ddot{x} + cx' + kx) = \mathcal{L}(F(t))$$

$$m(s^2 X(s) - s x(0) - \dot{x}(0)) + c(s X(s) - x(0)) + k X(s) = F(s)$$

$$X(s) (ms^2 + cs + k) = F(s) + x(0) (mst + c) + m \dot{x}(0)$$

Laplace take care of Initial Conditions automatically!

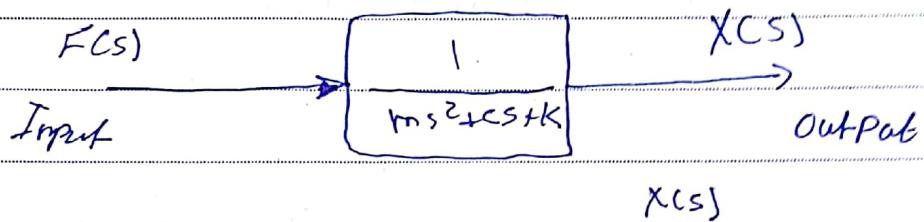
For Zero I.C's $\rightarrow X(0) = 0, \dot{X}(0) = 0$

$$X(s) = \frac{F(s)}{ms^2 + cs + k}$$

Transfer Function $\rightarrow \frac{\text{Output}}{\text{Input}}$

$$= \frac{X(s)}{F(s)} = \frac{1}{ms^2 + cs + k}$$

drawing the Block diagram:



after solving for the output back to time domain (Laplace Transform ~~then~~ (inverse))

$$x(t) = L^{-1}(X(s))$$

Final and Initial Value Theorem. (Steady State and)

this involves Complex integral

Usually we use tables and for linear system

\rightarrow any problem can be solved by Partial Fraction Expansion

Partial Fraction Example:

$$X(s) = \frac{s}{s^2 + 3s + 2}$$

حدايد Laplace inverse و تطبيق partial fraction expansion في حل

$$X(s) = \frac{s}{s^2 + 3s + 2} = \frac{a}{(s+1)} + \frac{b}{(s+2)}$$

وحدة مقاومات
عنوان البسط
يساوي البسط

$$\rightsquigarrow a = -17'$$

S_m = 2

$$-2 = 0 + -b$$

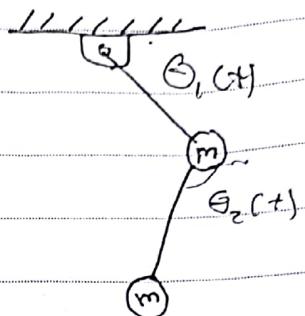
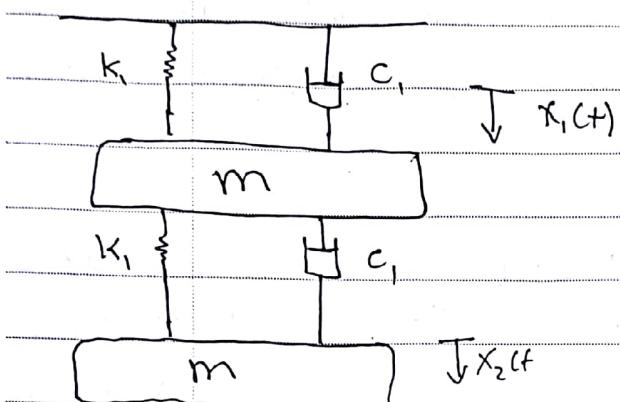
$$b = 2$$

$$\text{So } \mathcal{L} \left(\frac{-1}{s+1} + \frac{2}{s+2} \right) = -e^{-t} + 2e^{-2t}$$

"Solved easily"

Ch : Vibration of 2 D.o.F systems

Examples



Position Coordinates needed

→ The minimum number to describe the motion is 2.

→ the 2 coordinates are completely independent from each other, can't be related in an equation to convert it into a 5 DOF.

* For Free Vibration we are after "Natural Frequencies" =
and "modes" of Free vibration

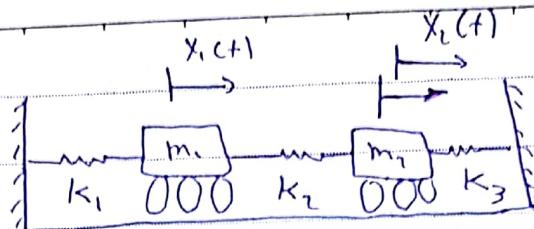
↳ (الخطوات الممكنة في الميكانيكا الحرارية)
(خطوات ممكنة)

Steps:

- 1 First we should (idealize) our system "Choose the coordinates"
- 2 apply physical laws to develop the E.O.M.
- 3 Perform to the resulting equations

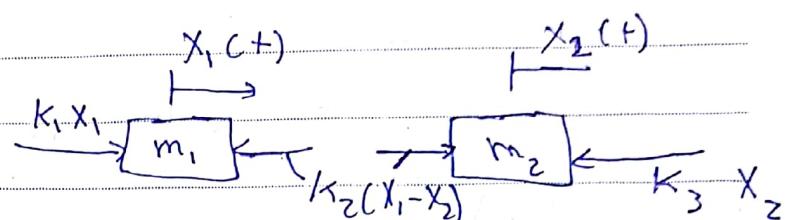
→ Natural Frequency and modes

Example:

Apply Newton's 2nd law for each of the mass at a time

F.B.D

- $x_1(t) > x_2(t)$

* For m_1 ,

$$\sum F_{x_1} = m_1 \ddot{x}_1 = -k_1 x_1 - k_2 (x_1 - x_2)$$

$$m_1 \ddot{x}_1 + (k_1 + k_2) x_1 - k_2 x_2 = 0 \quad \text{--- (1)}$$

* For m_2 :

$$\sum F_{x_2} = m_2 \ddot{x}_2 = k_2 (x_1 - x_2) - k_3 x_2$$

$$m_2 \ddot{x}_2 = + (k_2 + k_3) x_2 - k_2 x_1 \quad \text{--- (2)}$$

These 2 equations

Should be used together

to be solved

Re-arrange in matrix form Stiffness matrix

$$\begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} k_1+k_2 & -k_2 \\ -k_2 & k_2+k_3 \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

موديل معادل ديناميكي
هي ذات احوال ثابت

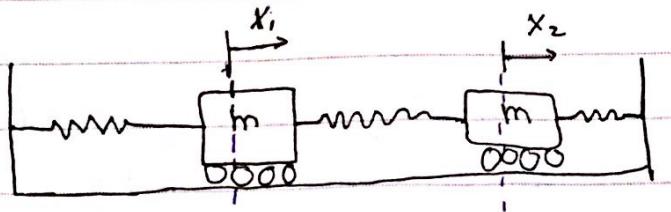
mass matrix

or

Inertia matrix

2 DoF Systems

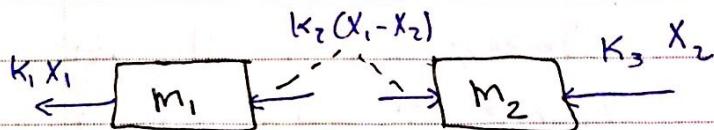
1- Undamped



* Free vibration (initial displacement)



* Free body diagram

Assumption $x_1 > x_2$ 

(العنصر الثاني من المهمة) (عنصر ثالث من المهمة)

Applying Newton's 2nd law :

$$(m_1) m_1 \ddot{x}_1 = -k_1 x_1 - k_2 (x_1 - x_2)$$

$$\hookrightarrow m_1 \ddot{x}_1 + k_1 x_1 + k_2 (x_1 - x_2) = 0$$

$$(m_2) m_2 \ddot{x}_2 = +k_2 (x_1 - x_2) - k_3 x_2$$

$$\hookrightarrow m_2 \ddot{x}_2 - k_2 (x_1 - x_2) + k_3 x_2 = 0$$

✓ Matrix \rightarrow Solution بقدر *

$$\begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 + k_3 \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

* In general, any two degree of freedom system

$$\begin{bmatrix} m_{ii} & m_{ij} \\ m_{ji} & m_{jj} \end{bmatrix} \begin{bmatrix} \ddot{x}_i \\ \ddot{x}_j \end{bmatrix} + \begin{bmatrix} k_{ii} & k_{ij} \\ k_{ji} & k_{jj} \end{bmatrix} \begin{bmatrix} x_i \\ x_j \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

if $m_{ij} = m_{ji}$ & $k_{ij} = k_{ji}$ \rightarrow Symmetric \rightarrow Positive Definite
 Determinate $\neq 0$ يعنى له معکوس
 Inverse موجود $\neq 0$ و يقدر

if $m_{jj} = 0 \rightarrow$ Dynamically uncoupled

$m_{ij} \neq 0 \rightarrow$ Dynamically Coupled

* Coupled \rightarrow اى انه الحركتين مرتبطتين معاً
 يعني في علاقة بينهم!

if $k_{ij} = 0 \rightarrow$ Statically uncoupled

$k_{ij} \neq 0 \rightarrow$ Statically Coupled

* For the previous example **

it is Dynamically uncoupled and
 Statically Coupled

Free Vibration Analysis :

- * Two Natural Frequencies
- * Two Mode Shapes (Natural Coordinate)

→ Eigen Value Problem

- * Assume motion to be harmonic.

$$x_1 = \underline{\underline{X}}_1 \sin(\omega t) \Rightarrow \ddot{x}_1 = -\underline{\underline{X}}_1 \omega^2 \sin(\omega t)$$

$$x_2 = \underline{\underline{X}}_2 \sin(\omega t) \Rightarrow \ddot{x}_2 = -\underline{\underline{X}}_2 \omega^2 \sin(\omega t)$$

Substitute in the Equation of Motion ...

عمر وحد

$$\left[-\omega^2 \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} + \begin{bmatrix} k_1+k_2 & -k_2 \\ -k_2 & k_2+k_3 \end{bmatrix} \begin{bmatrix} \underline{\underline{X}}_1 \\ \underline{\underline{X}}_2 \end{bmatrix} \right] \sin(\omega t) = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

فإذا كان $\underline{\underline{X}}_1$ و $\underline{\underline{X}}_2$ فاليساوي
عمر وحد ω معناتو القوس الثاني

doesn't equal
Zero

$$\begin{bmatrix} k_1+k_2-\omega^2 m_1 & -k_2 \\ -k_2 & k_2+k_3-\omega^2 m_2 \end{bmatrix} \begin{bmatrix} \underline{\underline{X}}_1 \\ \underline{\underline{X}}_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Eigen Value problem, Determinate = 0

$$\begin{vmatrix} (K_1 + K_2) - \omega^2 m_1 & -K_2 \\ -K_2 & (K_2 + K_3 - \omega^2 m_2) \end{vmatrix} = 0$$

$$\Rightarrow ((K_1 + K_2) - \omega^2 m_1) ((K_2 + K_3 - \omega^2 m_2) - K_2^2) = 0$$

$$\Rightarrow (K_1 + K_2) (K_2 + K_3) - (K_1 + K_2) \omega^2 m_2$$

$$-m_1 \omega^2 (K_2 + K_3) + \omega^4 m_2 m_1 - K_2^2 = 0$$

↓

$$m_1 m_2 \omega^4 - \omega^2 (m_1 (K_2 + K_3) + m_2 (K_1 + K_2))$$

$$+ (K_1 K_2 + K_1 K_3 + K_2 K_3) = 0$$

Assume $\lambda^2 = \omega^2$ \Rightarrow $\lambda = \pm \omega$ يعني (للحصول على معادلة تربيعية)

$$\rightarrow m_1 m_2 \lambda^2 - (m_1 (K_2 + K_3) + m_2 (K_1 + K_2)) \lambda$$

$$+ (K_1 K_2 + K_1 K_3 + K_2 K_3) = 0$$

$$\lambda_{1,2} = \frac{m_1(k_2+k_3) + m_2(k_1+k_2)}{2m_1m_2} \pm \sqrt{\frac{(m_1(k_2+k_3) + m_2(k_1+k_2))^2}{4m_1m_2} - 4m_1m_2(k_1k_2 + k_1k_3 + k_2k_1)}$$

القانون العام للعزم المائي

$$\lambda_1 = \omega_{n_1}^2 \rightarrow \omega_{n_1} = \sqrt{\lambda_1} \text{ rad/s}$$

$$\lambda_2 = \omega_{n_2}^2 \rightarrow \omega_{n_2} = \sqrt{\lambda_2} \text{ rad/s}$$

which are the two natural frequencies of the system

→ Now go back to eqn (*) solve for

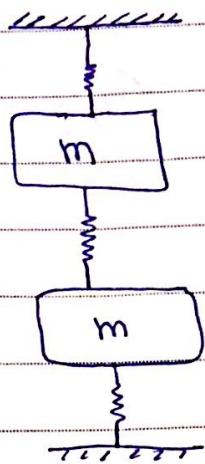
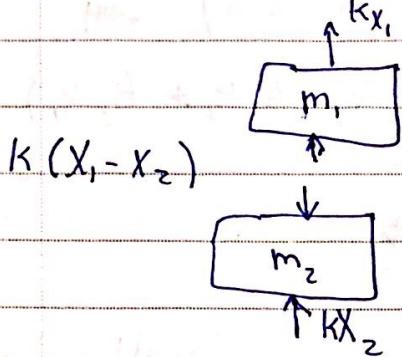
Vector which is the eigen vector $\begin{Bmatrix} x_1^{(1)} \\ x_2^{(1)} \end{Bmatrix}$
(Mode Shape)

Similarly, substitute for ω_{n_2} to solve for the eigen value which is the 2nd mode shape

Example

First we take the difflected Shape ..

and define the Forces acting on the 2 masses.



For the First mass : $\sum F_{x_1} = m \ddot{x}_1 = -Kx_1 - K(x_1 - x_2)$

$$\rightarrow m \ddot{x}_1 + Kx_1 + K(x_1 - x_2) = 0$$

For the Second mass : $\sum F_{x_2} = m \ddot{x}_2 = -Kx_2 + K(x_1 - x_2)$

$$m \ddot{x}_2 + Kx_2 + K(x_1 - x_2) = 0$$

$$m \ddot{x}_1 + 2Kx_1 - Kx_2 = 0$$

$$m \ddot{x}_2 + 2Kx_2 - Kx_1 = 0$$

$$\rightarrow \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} 2K & -K \\ -K & 2K \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

it is Eigen Value Problem ..

$$\left[[K] - \omega^2 [m] \right] [x] = 0$$

}

$$\begin{bmatrix} 2K - m\omega^2 & -K \\ -K & 2K - m\omega^2 \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = 0$$

$$\Delta(\omega^2) = \begin{vmatrix} 2K - m\omega^2 & -K \\ -K & 2K - m\omega^2 \end{vmatrix} = 0$$

$$(2K - m\omega^2)^2 - K^2 = 0$$

$$4K^2 - 4Km\omega^2 + m^2\omega^4 - K^2 = 0$$

$$m^2\omega^4 - 4Km\omega^2 + 3K^2$$

$$\text{assume } \omega^4 = \gamma^2$$

$$m^2\gamma^2 - 4Km\gamma + 3K^2 = 0$$

roots

equation

det.

ω_{n1} & ω_{n2}

$$\omega^2 = \frac{km \pm \sqrt{16k^2m^2 - 12m^2k^2}}{2m^2}$$

$$= \frac{4km \pm 2mk}{2m^2}$$

$$\omega_2^2 = \frac{6k}{2m} = \frac{3k}{m} \Rightarrow \omega_2 = \sqrt{\frac{3}{2} \frac{k}{m}}$$

$$\omega_1^2 = \frac{2km}{2m^2} = \frac{k}{m} \Rightarrow \omega_1 = \sqrt{\frac{k}{m}}$$

~~~~~

Recall  $(2k - m\omega^2)x_1 - kx_2 = 0$

Substitute by  $\omega_1$ , and define  $r_1 = \frac{x_1}{x_2}$

$$r_1 = \frac{x_1}{x_2} = \frac{k}{2k - m\omega_1^2} = \frac{k}{2k - m\frac{k}{m}} = 1$$

مقدار الارجح يعني

$\omega_1$  ينتمي مع

$$\xrightarrow{\sim} \begin{bmatrix} x_1^{(1)} \\ x_2^{(1)} \end{bmatrix} = \begin{bmatrix} x_1^{(1)} \\ \frac{x_1^{(1)}}{r_1} \end{bmatrix} = \begin{bmatrix} 1 \\ 1 \end{bmatrix}$$

$m_1$  ↑ مقدار يعني يتحرك مع  $x_2$  مع  $x_1$   
 $m_2$  ↑

For the 2<sup>nd</sup> natural frequency.

$$(2K - m\omega_2^2) X_1^{(2)} - K X_2^{(2)} = 0$$

$$r_2 = \frac{X_2^{(2)}}{X_1^{(2)}} = \frac{2K - m\omega_2^2}{K} = \frac{2K - m \frac{3K}{m}}{K} = \frac{-K}{K} = -1$$

$$\begin{bmatrix} X_1^{(2)} \\ X_2^{(2)} \end{bmatrix} = \begin{bmatrix} X_1 \\ r_2 X_1 \end{bmatrix} = X_1 \begin{bmatrix} 1 \\ -1 \end{bmatrix}$$

$m_1 \downarrow$        $m_2 \uparrow$       يعني بمحض ما بعد  $r_2$  ينبع حركة  $m_2$  بعكس حركة  $m_1$

First mode

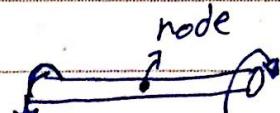
$$m_1 = m_2 \text{ لأن المasses متساويات}$$

$$\omega_1 = \sqrt{\frac{K}{m}}$$

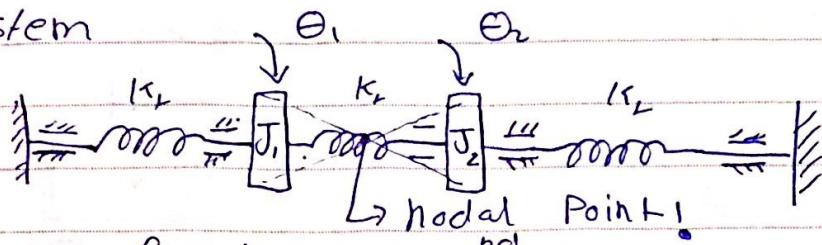
Second mode

$$\omega_2 = \sqrt{\frac{3K}{2m}}$$

(No motion)      }      \* Rule:      NO.  
 (No Deflection)      }      NO. of nodes = Mode - 1  
 Torsion example      }      For each mode

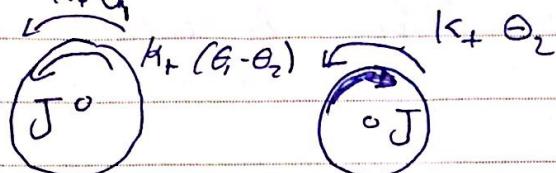


## Torsional System



node انجيلها ادار  
يتغير على الى السما  
كبير J

For Newtons 2<sup>nd</sup> law



(+)

$$\sum M_{J_1} = J_1 \ddot{\theta}_1 = -K_1 \theta_1 - K_t (\theta_1 - \theta_2) = 0$$

$$J_1 \ddot{\theta}_1 + 2K_1 \theta_1 - K_t \theta_2 = 0$$

$$\sum M_{J_2} = J_2 \ddot{\theta}_2 = -K_t \theta_2 + K_t (\theta_1 - \theta_2)$$

$$J_2 \ddot{\theta}_2 = -K_t \theta_2 + K_t (\theta_1 - \theta_2)$$

$$J_2 \ddot{\theta}_2 + 2K_t \theta_2 - K_t \theta_1 = 0$$

so

$$\begin{bmatrix} J_1 & 0 \\ 0 & J_2 \end{bmatrix} \begin{bmatrix} \ddot{\theta}_1 \\ \ddot{\theta}_2 \end{bmatrix} + \begin{bmatrix} 2K_1 & -K_t \\ -K_t & 2K_t \end{bmatrix} \begin{bmatrix} \theta_1 \\ \theta_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Same as Previous example!

$$\omega_1 = \sqrt{\frac{K_t}{J}} \quad , \quad \omega_2 = \sqrt{\frac{3K_t}{J}}$$

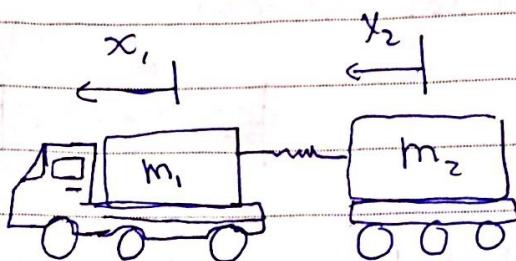
$$\text{Mode 1} = \begin{bmatrix} 1 \\ 1 \end{bmatrix}$$

and Same nodes

$$\text{Mode 2} = \begin{bmatrix} 1 \\ -1 \end{bmatrix}$$

Example ..

homework

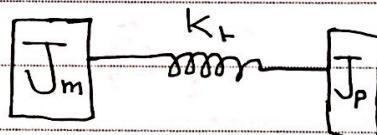


Find the Natural Frequencies and mode Shapes.

\* From the Solution, we can see that  $\omega_n = 0$ ,  $T_{\text{period}} = \infty$

which means that there is no vibration..

and this happen in torsional systems as in pumps and motor.



$$\begin{bmatrix} J_m & 0 \\ 0 & J_p \end{bmatrix} \begin{bmatrix} \ddot{\theta}_1 \\ \ddot{\theta}_2 \end{bmatrix} + \begin{bmatrix} k_r & -k_r \\ -k_r & k_r \end{bmatrix} \begin{bmatrix} \theta_1 \\ \theta_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

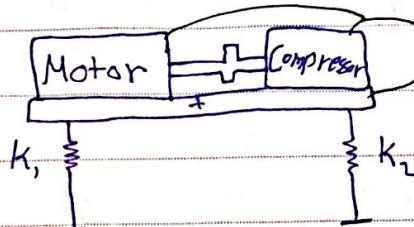
$$\Delta k = 0 !!$$

Due to rigid body mode...

any 2DoF can be represented in a matrix

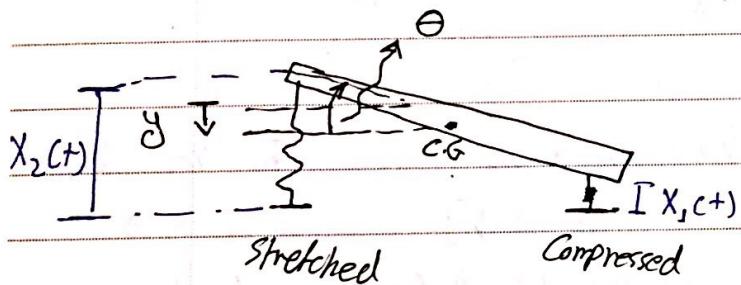
$$\begin{bmatrix} m_{11} & m_{12} \\ m_{21} & m_{22} \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

### Example:



assuming the bodies are Rigid body

First  $\rightarrow$  What are the degree of freedom?  $\rightarrow 2$



\* ينحدر أوجه الحركة بـ  
١) (حركة الـ G.C يشكل  
عمودي )

Rotational Motion  $\leftarrow \theta$  (c)  
C. G حول اد

أ و بنفس الوقت يقدر أو حفظ ما هي المركبة بـ  $x_1(t)$  &  $x_2(t)$  و هي  $2 \text{ DoF}$  ..

## لطفی آخر

There must be some coordinate system  $q_1(t)$  &  $q_2(t)$  that we can use and result a new system of equation (even if there is no physical meaning) that will lead to uncoupled system which mean the two equation can be solved independently. These

Coordinates are the Natural (Principle) Coordinates

\* Uncoupled ns

يعني ماتم تخلص من

matrix وينتظر اد

والمسمى بالطبقة

$$\begin{bmatrix} m_{qq_1} & 0 \\ 0 & m_{qq_2} \end{bmatrix} \begin{bmatrix} \ddot{q}_1 \\ \ddot{q}_2 \end{bmatrix} + \begin{bmatrix} k_{qq_1} & 0 \\ 0 & k_{qq_2} \end{bmatrix} \begin{bmatrix} q_1 \\ q_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

}

$$m_{qq_1} \ddot{q}_1 + k_{qq_1} q_1 = 0 \quad q_1(+) = ?$$

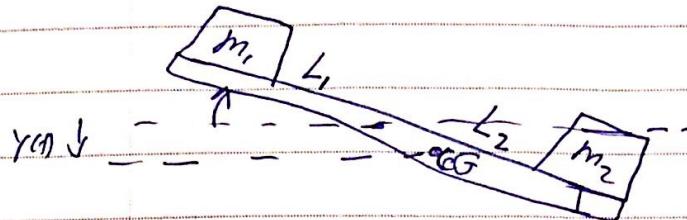
Independent

$$m_{qq_2} \ddot{q}_2 + k_{qq_2} q_2 = 0 \quad q_2(+) = ?$$

Now there must be a transformation that can transform  
 $[M]$  and  $[K]$  from  $[X]$  to  $[q]$

i.e. from physical coordinates to natural coordinates ...

Example: recall the motor and compressor and solve it with  $g(t)$  and  $\theta(t)$



$$x_1 = L_1 \theta - y$$

$$x_2 = y + L_2 \theta$$

Apply Newton's 2<sup>nd</sup> law

homework → find the Equation of motion

Recall the 2 DoF Free vibration Matrix :

$$\begin{bmatrix} m_{11} & m_{12} \\ m_{21} & m_{22} \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

→ Eigen Value problem →  $\omega_1, \omega_2$

$$\begin{bmatrix} x_1^{(1)} & x_1^{(2)} \\ x_2^{(1)} & x_2^{(2)} \end{bmatrix} \rightarrow \text{Modal Matrix } [V]$$

$$\Rightarrow [m_{\text{new}}] = [V]^T [m] [V]$$

↑ transposed

{  $[V]^T \circ V \rightarrow \text{Identity Matrix}$   
 Diagonal Matrix  $\rightarrow$  Uncoupled equations.

$$= \begin{bmatrix} m_{1\text{new}} & 0 \\ 0 & m_{2\text{new}} \end{bmatrix}$$

Similarly for  $K$

$$[k_{\text{new}}] = [V]^T [ ] [ ]$$

Swap between Rows and Columns

$$= \begin{bmatrix} k_{1\text{new}} & 0 \\ 0 & k_{2\text{new}} \end{bmatrix}$$

that will give Modal equations that are uncoupled where each of them can be solved independently from the other with new coordinate  $\{q_1, q_2\}$  and are called the natural coordinates



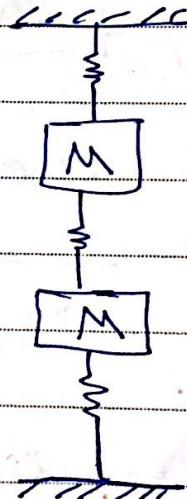
$$\begin{bmatrix} m_{1\text{new}} & 0 \\ 0 & m_{2\text{new}} \end{bmatrix} \begin{bmatrix} \ddot{q}_1 \\ \ddot{q}_2 \end{bmatrix} + \begin{bmatrix} k_{1\text{new}} & 0 \\ 0 & k_{2\text{new}} \end{bmatrix} \begin{bmatrix} q_1 \\ q_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

$$\Rightarrow \text{Uncoupled} \Rightarrow m_{1\text{new}} \ddot{q}_1 + k_{1\text{new}} q_1 = 0$$

$$m_{2\text{new}} \ddot{q}_2 + k_{2\text{new}} q_2 = 0$$

\* this can be applied to any undamped multi-degree of freedom systems (check)

H.W on Wednesday (Very Important to be Studied and Solved)

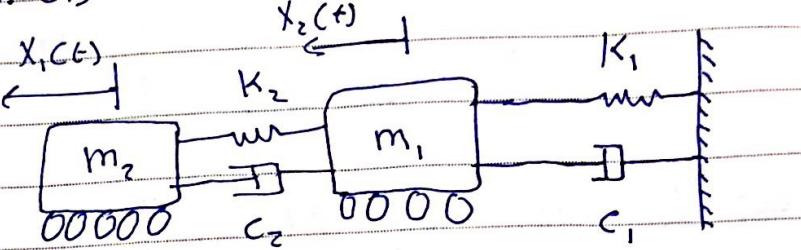


- 1 Formulate Equation of motion
- 2 Find natural frequencies
- 3 Find modal matrix
- 4 Uncouple equations
- 5 get final uncoupled equations

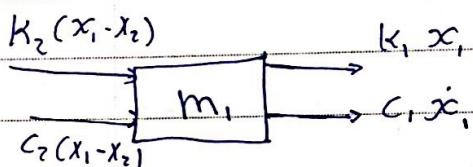
|                       |
|-----------------------|
| $k = 100 \text{ N/m}$ |
| $m = 1 \text{ kg}$    |

## \* Damped 2 D.O.F System

→ Formulate the damped  
Equation of motion  
→ Newton's Laws



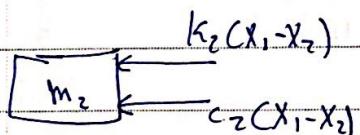
*motion for  
light damping  
and with  
initial disturbance*



### \* Assumptions:

- Linear Springs
- Linear dampers
- Small motion

and



So the Equation of motion

→ Apply Newton's 2<sup>nd</sup> law

$$\sum F_{x_1} = m_1 \ddot{x}_1 = -k_1 x_1 - c_1 \dot{x}_1 - k_2(x_1 - x_2) - c_2(x_1 - \dot{x}_2)$$

$$\Rightarrow m_1 \ddot{x}_1 + (c_1 + c_2) \dot{x}_1 + c_2 \dot{x}_2 + (k_1 + k_2) x_1 - k_2 x_2 = 0$$

$$\sum F_{x_2} = m_2 \ddot{x}_2 = c_2(x_1 - \dot{x}_2) + k_2(x_1 - x_2)$$

$$\Rightarrow m_2 \ddot{x}_2 + c_2 \dot{x}_2 - c_2 \dot{x}_1 + k_2 x_2 - k_2 x_1 = 0$$

the matrix for these 2 equation is

$$\underbrace{\begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix}}_{\text{mass matrix}} + \underbrace{\begin{bmatrix} c_1 + c_2 & -c_2 \\ -c_2 & c_2 \end{bmatrix} \begin{bmatrix} \dot{x}_1 \\ \dot{x}_2 \end{bmatrix}}_{\text{damping matrix}} + \underbrace{\begin{bmatrix} k_1 + k_2 & -k_1 \\ -k_2 & k_2 \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix}}_{\text{Stiffness matrix}} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Now what is the Eigen Value problem for this system?

Recall  $\rightarrow$  Eigen Value  $\rightarrow$  Natural frequencies

Eigen Vector  $\rightarrow$  Mode shapes..

$x_1 e^{j\omega t} - x_2$  complex con Damping  $\rightarrow$   $\omega$  is  $\omega_n$  (natural frequency)  $\rightarrow$  Imaginary and Real number  $\omega_n$   $\rightarrow$  First order  $\rightarrow$  2<sup>nd</sup> order  $\rightarrow$   $\omega_n$  حول  $\omega$  فلك

$\rightarrow$  to find eigen Values / eigen Vectors  $\Rightarrow$  1<sup>st</sup> order System

$$[m] [\ddot{x}] + [c] [\dot{x}] + [k] [x] = [0]; \quad x = \begin{bmatrix} x_1 \\ x_2 \end{bmatrix}$$

$$\begin{cases} y_1, \text{new} \\ y_2, \text{new} \end{cases} \rightarrow \text{new Coordination System such that } y_1 = x_1, \quad y_2 = \dot{x}_1, \quad y_3 = x_2, \quad y_4 = \dot{x}_2$$

$$y_1 = \ddot{x}_1 \quad \leftarrow \quad y_4 = \ddot{x}_2$$

\*  $[m^{-1}] * [m] \rightsquigarrow$  Identity (1)

$[m]^{-1} \rightarrow$  initial \*

$$\rightsquigarrow [\ddot{x}] + [m]^{-1} [c] \{ \dot{x}_1 \} + [m]^{-1} [k] \{ x_2 \} = \{ 0 \}$$

{ }

$$\dot{y}_1 = y_2$$

$$\dot{y}_3 = y_4$$

~~dot~~

$$\begin{bmatrix} \dot{y}_1 \\ \dot{y}_2 \\ \dot{y}_3 \\ \dot{y}_4 \end{bmatrix} = \begin{bmatrix} I & 0 \\ -[m]^{-1} [k] & -[m]^{-1} [c] \end{bmatrix} \begin{bmatrix} y_1 \\ y_2 \\ y_3 \\ y_4 \end{bmatrix}$$

↓

$$[A]$$

Check the Numerical Example \*

## Forced Vibration (Harmonic excitation)

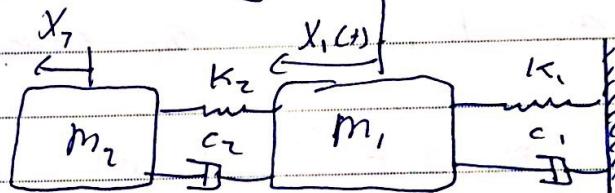
$$\begin{bmatrix} m_{11} & m_{12} \\ m_{21} & m_{22} \end{bmatrix} \begin{bmatrix} \ddot{x}_1 \\ \ddot{x}_2 \end{bmatrix} + \begin{bmatrix} c_{11} & c_{12} \\ c_{21} & c_{22} \end{bmatrix} \begin{bmatrix} \dot{x}_1 \\ \dot{x}_2 \end{bmatrix} + \begin{bmatrix} k_{11} & k_{12} \\ k_{21} & k_{22} \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} F_0 e^{j\omega t} \\ 0 \end{bmatrix}$$

$\omega$  is the forcing frequency

$$F_0 e^{j\omega t} \begin{bmatrix} F_0 e^{j\omega t} \\ 0 \end{bmatrix}$$

and output follows the input

$$x_1(t) = x_1 e^{j\omega t}$$



$$x_2(t) = x_2 e^{j\omega t} \quad \text{and} \quad \dot{x}_1 = x_1 j\omega e^{j\omega t} \quad \ddot{x}_1 = -x_1 \omega^2 e^{j\omega t}$$

$$\dot{x}_2 = x_2 j\omega e^{j\omega t} \quad \ddot{x}_2 = -x_2 \omega^2 e^{j\omega t}$$

$\Downarrow$  ~~مُعَدِّلِيَّاتِ جَهَنَّمِيَّاتِ~~  $e^{j\omega t}$  ~~مُعَدِّلِيَّاتِ~~  $x_1$  &  $x_2$  ~~مُعَدِّلِيَّاتِ~~

$$\begin{bmatrix} k_{11} - m_{11}\omega^2 + j c_{11}\omega & k_{12} - m_{12}\omega^2 + j c_{12}\omega \\ k_{21} - m_{21}\omega^2 + j c_{21}\omega & k_{22} - m_{22}\omega^2 + j c_{22}\omega \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} F_0 \\ 0 \end{bmatrix}$$

$$Z_{rs} = k_{rs} - m_{rs}\omega^2 + j c_{rs}\omega \quad \text{عن طريق جعل$$

$$\begin{bmatrix} Z_{11} & Z_{12} \\ Z_{21} & Z_{22} \end{bmatrix} \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = \begin{bmatrix} F_0 \\ 0 \end{bmatrix} \Rightarrow \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = [Z]^{-1} \begin{bmatrix} F_0 \\ 0 \end{bmatrix}$$

وهي موجود

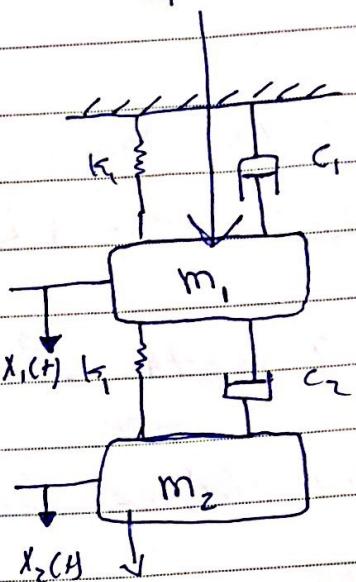
$\rightarrow$  Mechanical Impedance

## Harmonic Excitation (Steady State)

 $F_0 \cos(\omega t)$ 

Another example:

and we assumed that the output follows

the input  $\rightarrow X_1(t) = X_1 (i\omega) \cos(\omega t)$  $X_2(t) = X_2 (i\omega) \cos(\omega t)$ 

and reached for the impedance matrix

$$\begin{bmatrix} Z_{11} & Z_{12} \\ Z_{21} & Z_{22} \end{bmatrix} \begin{bmatrix} X_1 (i\omega) \\ X_2 (i\omega) \end{bmatrix} = \begin{bmatrix} F_0 \\ 0 \end{bmatrix}$$

{

$$\begin{bmatrix} X_1 (i\omega) \\ X_2 (i\omega) \end{bmatrix} = [Z^{-1}] \begin{bmatrix} F \end{bmatrix}$$

$$\rightarrow Z_{rs} = k_{rs} - m_{rs} \omega^2 + i C_{rs} \omega$$

$$\rightarrow [Z]^{-1} = \frac{1}{Z_{12}^2 - Z_{11}Z_{22}} \begin{bmatrix} Z_{22} & -Z_{12} \\ -Z_{12} & Z_{11} \end{bmatrix}$$

1

دلي اللى

Determinant

$$X_1(iw) = \frac{Z_{22} F_{01}}{Z_{12}^2 - Z_{11} Z_{22}}$$

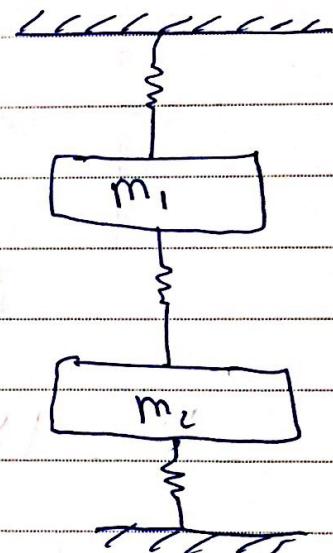
and

$$X_2(iw) = \frac{-Z_{12} F_{01}}{Z_{12}^2 - Z_{11} Z_{22}}$$

Example 8 (مثال محاول بالكتاب)  
(homework exercise)

Solution:

$$m_{12} = m_{21} = 0 \quad \left\{ \begin{array}{l} C_{ij} = 0 \end{array} \right.$$



$$m_{11} = m_{22} = m \quad \left\{ \begin{array}{l} k_{11} = k_{22} = 2k \\ k_{12} = k_{21} = -k \end{array} \right.$$

the impedance is

$$Z_{11} = 2k - m\omega^2$$

$$Z_{12} = Z_{21} = -k$$

$$Z_{22} = 2k - m\omega^2$$

$$\text{So } \Rightarrow X_1(iw) = \frac{(2k - m\omega^2) F_{01}}{k^2 - (2k - m\omega^2)^2} \quad \left\{ \begin{array}{l} \omega_1 = \sqrt{\frac{k}{m}} \\ \omega_2 = \sqrt{\frac{3k}{m}} \end{array} \right.$$

$$X_2(iw) = \frac{k F_{01}}{k^2 - (2k - m\omega^2)^2}$$

Mech Family

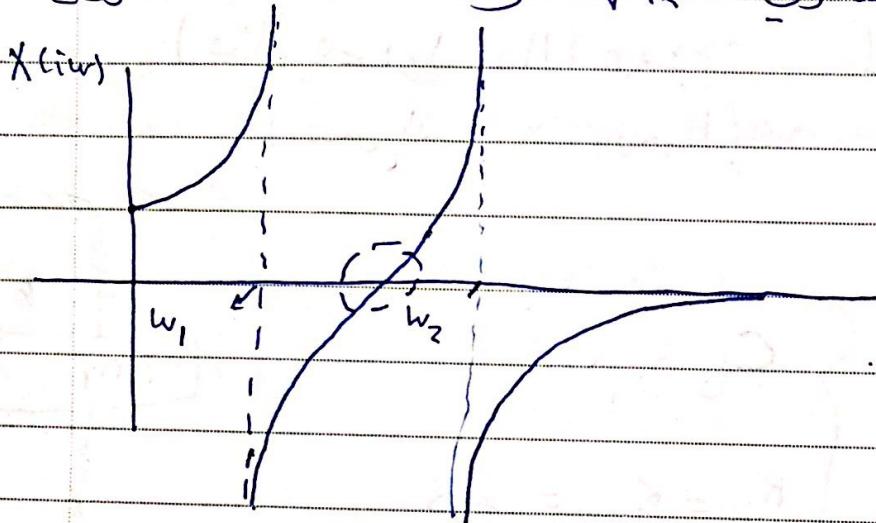
بدنا نوصلها ... حلينا انتشوف المختام

$$k^2 - (2k - m\omega^2) = 0 \Rightarrow \omega = \sqrt{\frac{2k}{m}} \rightarrow \infty \text{ لا يجوز ذلك}$$

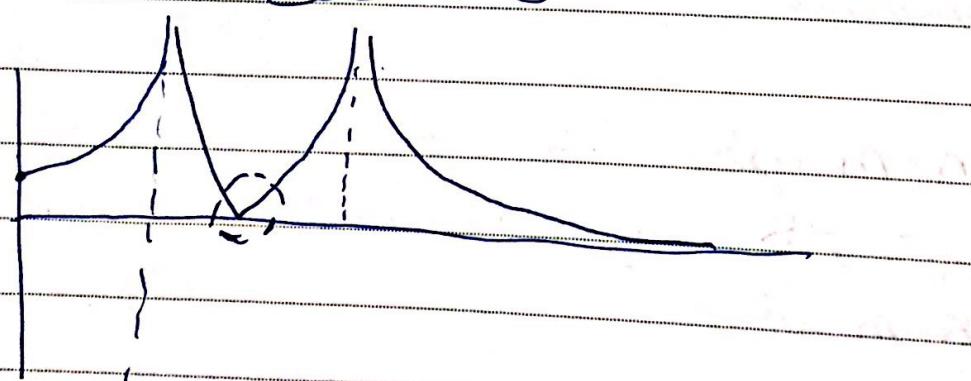
and

$$\omega = \sqrt{\frac{3k}{m}}$$

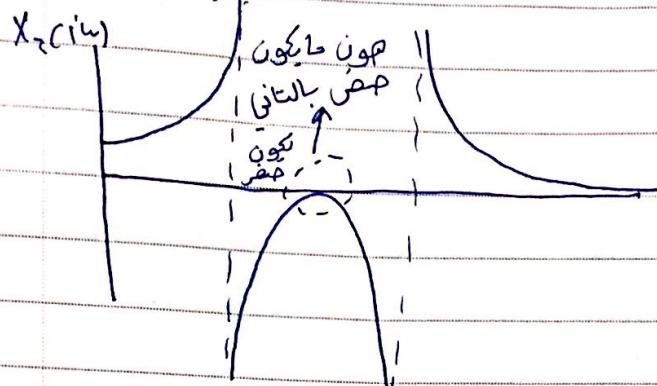
$\infty$  لا يجوز ذلك  $\omega = \sqrt{\frac{3k}{m}}$  هو  $\sqrt{\frac{k}{m}}$  يساوي  $\omega_0$  الباقي



$\omega_0$  اذن كانت  $|x_1|$  (محلق)



For  $X_2(\omega)$



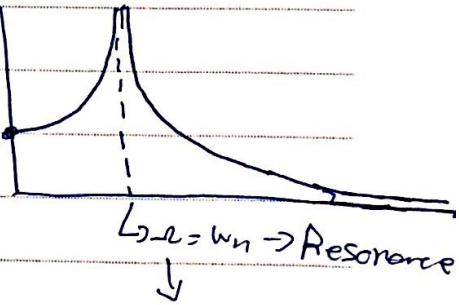
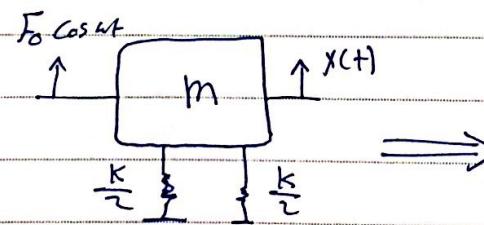
\* أهم ملخص بجهود الرسبي انه في منطقة بينيه قيمة  $\omega_2$  تساوي دفعه سعى ودون دفعه اثنى يستعمل اسلوب

### Dynamic Vibration Absorber idea

\* من خلال اني أتعلق mass وعندما ال  $\omega_2$  اللي يستعمل عليها وبتحفيزي ابداً

Chapter 9: Vibration Control \* هاد اتجاه موجود بـ

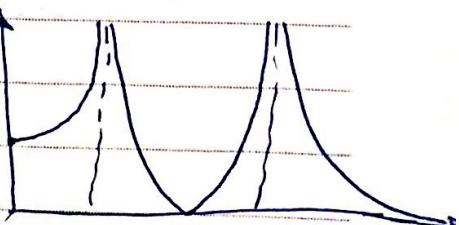
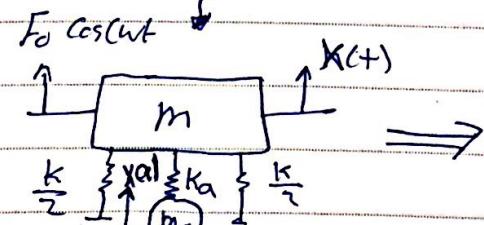
\* Example



بدى أتعلق ال

2 DoF مع عى بـ

بكون بـ  $m_a$  بـ  $\omega_a$



هيل هاد عى  $m_a$  ويغار عن اللي بدوى الـ  $\omega_a$  + الصغر اللي أنا شخاع عى ...

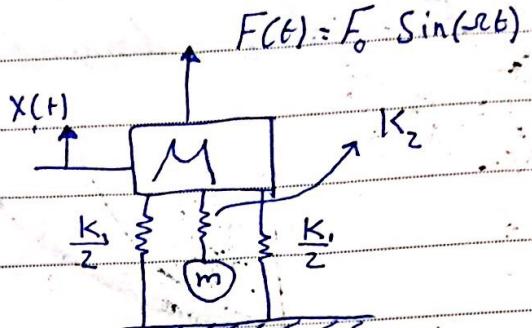
\* if Absorber Frequency is tuned to be equal to the external excitation Frequency  $\omega_0$  Amplitude will remain Primary

Check the Doctor paper (Dual dynamic absorber 1999)

## Dynamic absorber

→ For undamped System

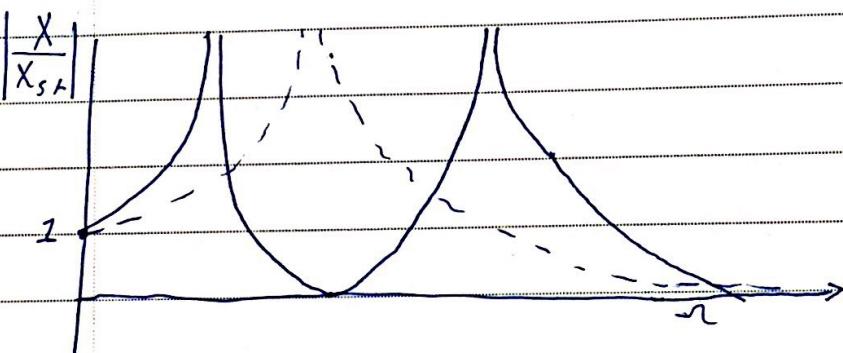
$$m_1 \ddot{x}_1 + K_1 x_1 = F_0 \sin(\omega t)$$



\* if we add this

where  $\omega$  is the forcing frequencymass, the System  
become 2 DoF

$$\omega \rightarrow \omega_1 \rightarrow \omega_1 = \sqrt{\frac{K}{m}}$$



$$m_1 \ddot{x}_1 + K_1 x_1 + K_2 (x_2 - x_1) = F_0 \sin(\omega t)$$

$$m_2 \ddot{x}_2 + K_2 (x_2 - x_1) = 0$$

$$m_1 (-\omega^2 x_1 \sin(\omega t)) + K_1 x_1 \sin(\omega t)$$

$$+ K_2 (x_2 - x_1) \sin(\omega t) = F_0 \sin(\omega t)$$

the Steady state response:

$$x_1(t) = X_1 \sin \omega t$$

So:

$$X_1 = \frac{(K_2 - m_2 \omega^2) F_0}{\Delta}$$

$$\ddot{x}_1 = -\omega^2 X_1 \sin \omega t$$

&amp;

$$\ddot{x}_2 = -\omega^2 X_2 \sin \omega t$$

$$X_2 = \frac{K_2 F_0}{\Delta}$$

ملاحظة 1: (عذر)

# Dual Dynamic absorber

No. ....

\* after adding  $(m_a, k_a)$  System become 2 DoF System

→ Vibration is a form of Energy...

So it will go to the System even if it is close to resonance (by the easiest way, which mean to the added mass)

\* If absorber Frequency is tuned to be equal to the external excitation Frequency, so the amplitude will remain at primary.

